

APPENDICES

Appendix A: Existing structural plans

Appendix B: Hand Calculations

Appendix C: New floor plan designs

**Complete hand calculations and computer output are available upon request

APPENDIX A: EXISTING STRUCTURAL PLANS



ALTICOR



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GENERAL CONTRACTOR
FSHBECK CONSULTING GROUP/FISHER, INC.

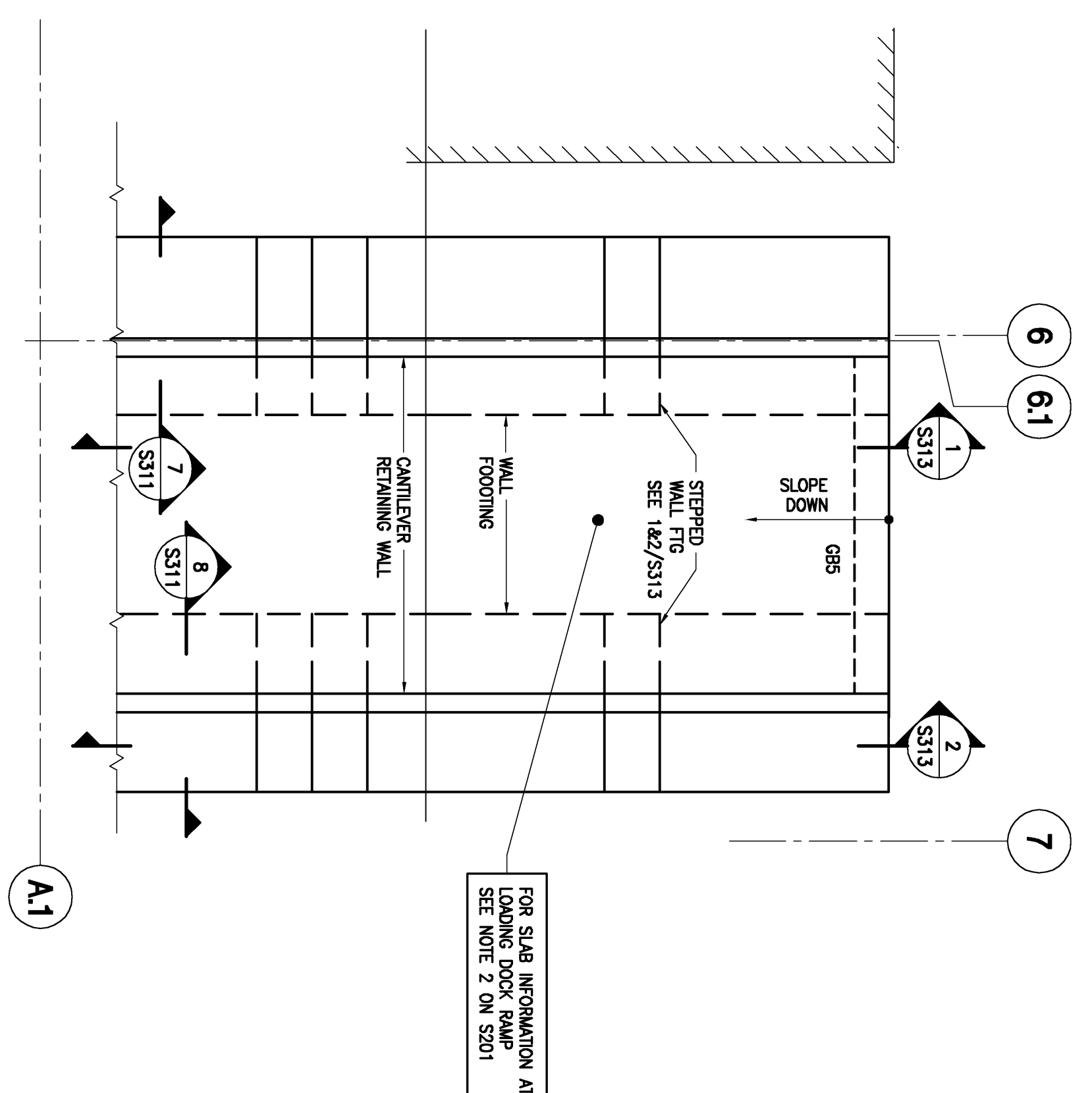
1515 Acorn Drive, SE, Grand Rapids, Michigan 49506
Tel: 616.455.1100 www.fishbeck.com

GENERAL CONTRACTOR
DANIEL WEINBACH & PARTNERS, LTD.

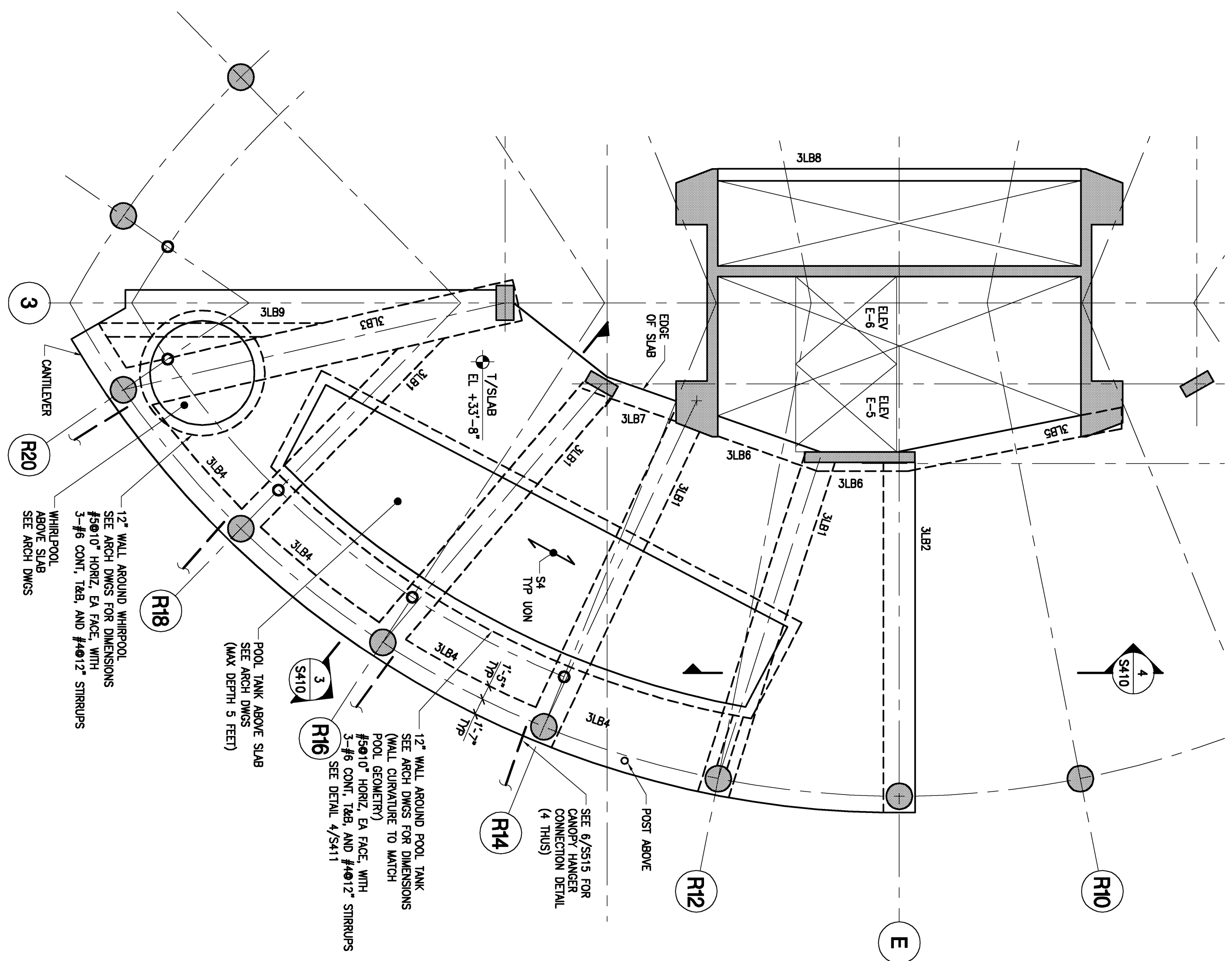
1000 East Fulton Street, Grand Rapids, MI 49504
Tel: 616.312.6724 www.danielweinbach.com



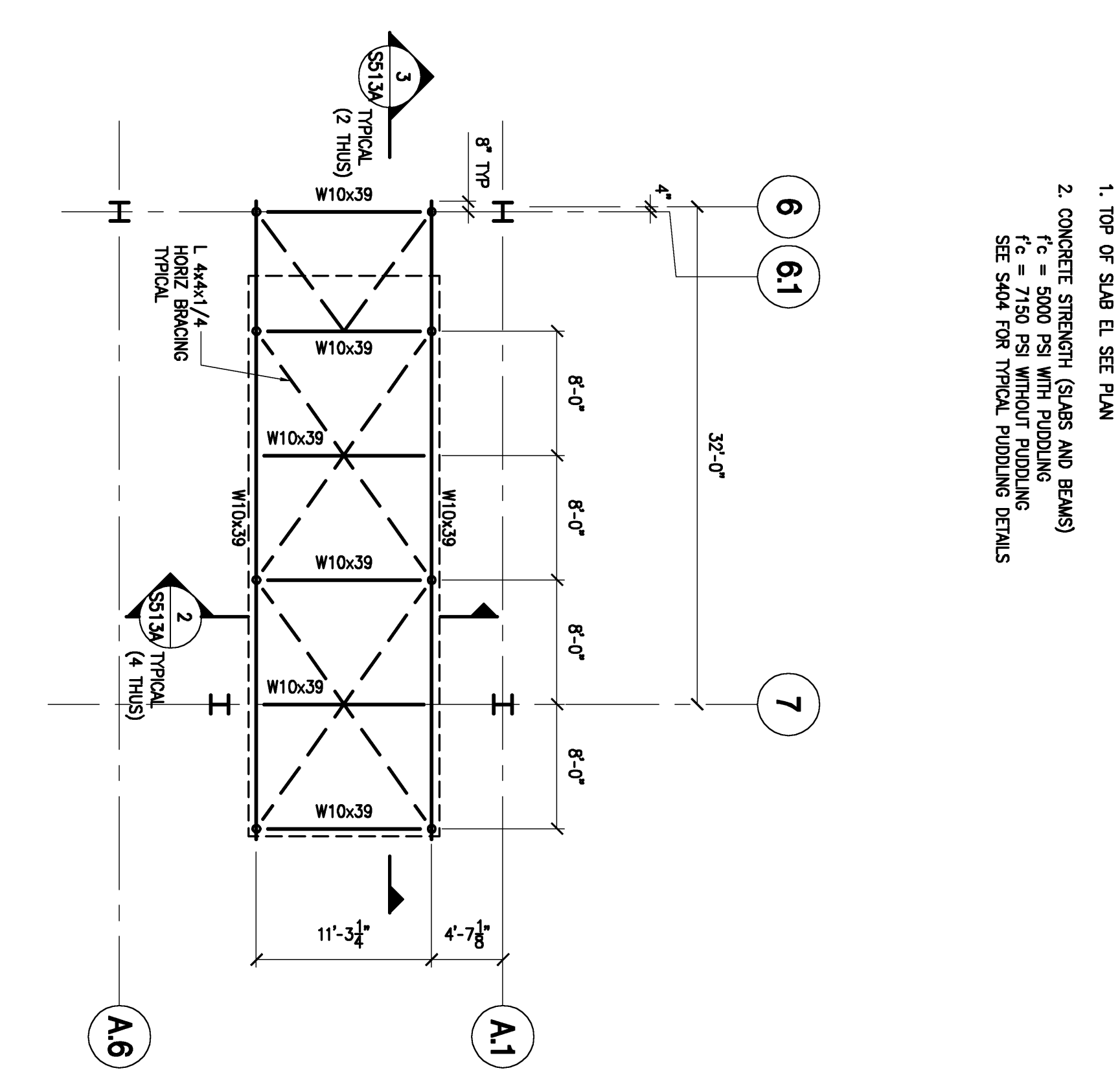
ALTIOR



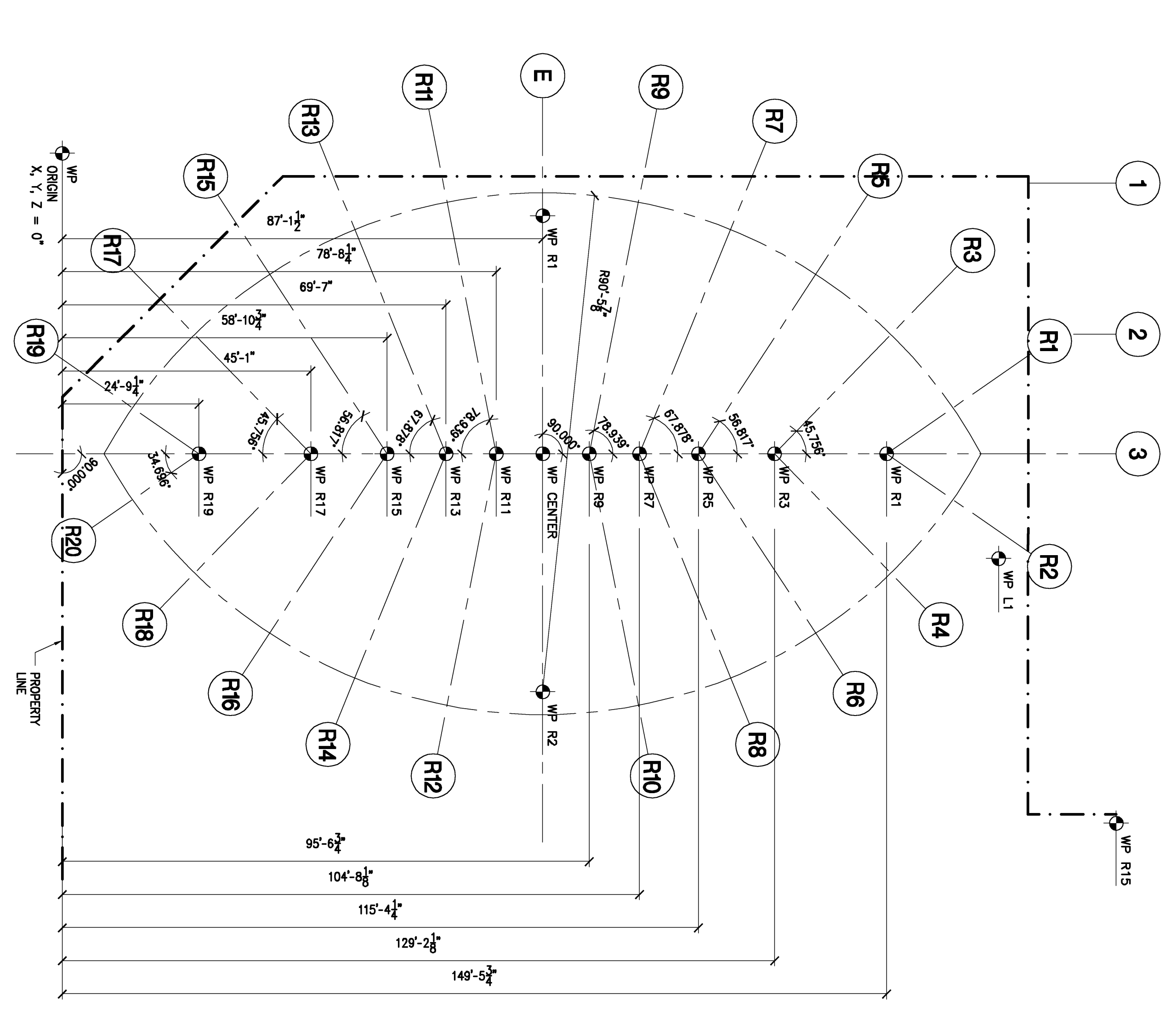
1 PARTIAL PLAN
SCALE: 1/8"=1'-0"
NOTES:
1. SEE S201 FOR ADDITIONAL INFORMATION



3 PARTIAL FRAMING PLAN
SCALE: 1/8"=1'-0"
NOTES:
1. TOP OF SLAB IS SEE PLAN
2. CONCRETE STRENGTH (SLABS AND BEAMS) f'c = 5000 PSI WITH PRODUCE f'c = 2750 PSI WITH PRODUCE SEE S201 FOR TYPICAL PRODUCTION DETAILS



4 PLATFORM FRAMING PLAN
SCALE: 1/8"=1'-0"
NOTES:
1. TOP OF STEEL IS 4'-3 1/2"±
2. ALL DIMENSIONS AND ELEVATIONS MUST BE VERIFIED BY ELEVATOR MANUFACTURER.
3. REFER TO EQUIPMENT DRAWINGS FOR MOUNTING HOLE LOCATIONS



2 TOWER GEOMETRY
SCALE: 1/16"=1'-0"
NOTES:
1. SEE S201 FOR ADDITIONAL INFORMATION

NO.	DATE	DESCRIPTION
17	04 DEC 05	ISSUED FOR CONSTRUCTION AND PERMIT
16	28 OCT 05	OWNER AND MARRIOTT CD REVIEW
15	24 OCT 05	OWNER CONCEPTS 80% PRODUCE
14	21 OCT 05	FINAL DESIGN REVIEW
13	20 OCT 05	FINAL DESIGN REVIEW
12	18 OCT 05	FINAL DESIGN REVIEW
11	15 OCT 05	FINAL DESIGN REVIEW
10	12 OCT 05	FINAL DESIGN REVIEW
9	09 OCT 05	FINAL DESIGN REVIEW
8	06 OCT 05	FINAL DESIGN REVIEW
7	03 OCT 05	FINAL DESIGN REVIEW
6	30 SEP 05	FINAL DESIGN REVIEW
5	27 SEP 05	FINAL DESIGN REVIEW
4	24 SEP 05	FINAL DESIGN REVIEW
3	21 SEP 05	FINAL DESIGN REVIEW
2	18 SEP 05	FINAL DESIGN REVIEW
1	15 SEP 05	FINAL DESIGN REVIEW

PARTIAL FRAMING PLANS

PROJECT LOCATION: 400' W. GRAND AVENUE + 415' S. 200TH LORAIN CANAL DISTRICT GRAND RAPIDS, MI
SCALE: AS SHOWN
DRAWN BY: JAS SHOWN
CHECKED BY: JAS SHOWN
PROJECT NO.: CCM48700
DRAWN NUMBER:

S2666

NO.	DATE	DESCRIPTION
17	02 DEC 05	ISSUED FOR CONSTRUCTION AND PERMIT
16	28 OCT 05	OWNER AND MARIOTT CD REVIEW
15	24 OCT 05	OWNER CONCRETE BID PACKAGE
9	03 AUG 05	STRUCTURAL STEEL BID PACKAGE
5	29 MAY 05	REVISED DESIGN DEVELOPMENT
3	29 APR 05	DESIGN DEVELOPMENT
2	15 APR 05	INTERIM DESIGN DEVELOPMENT
1	10 DEC 04	CONCEPT DESIGN

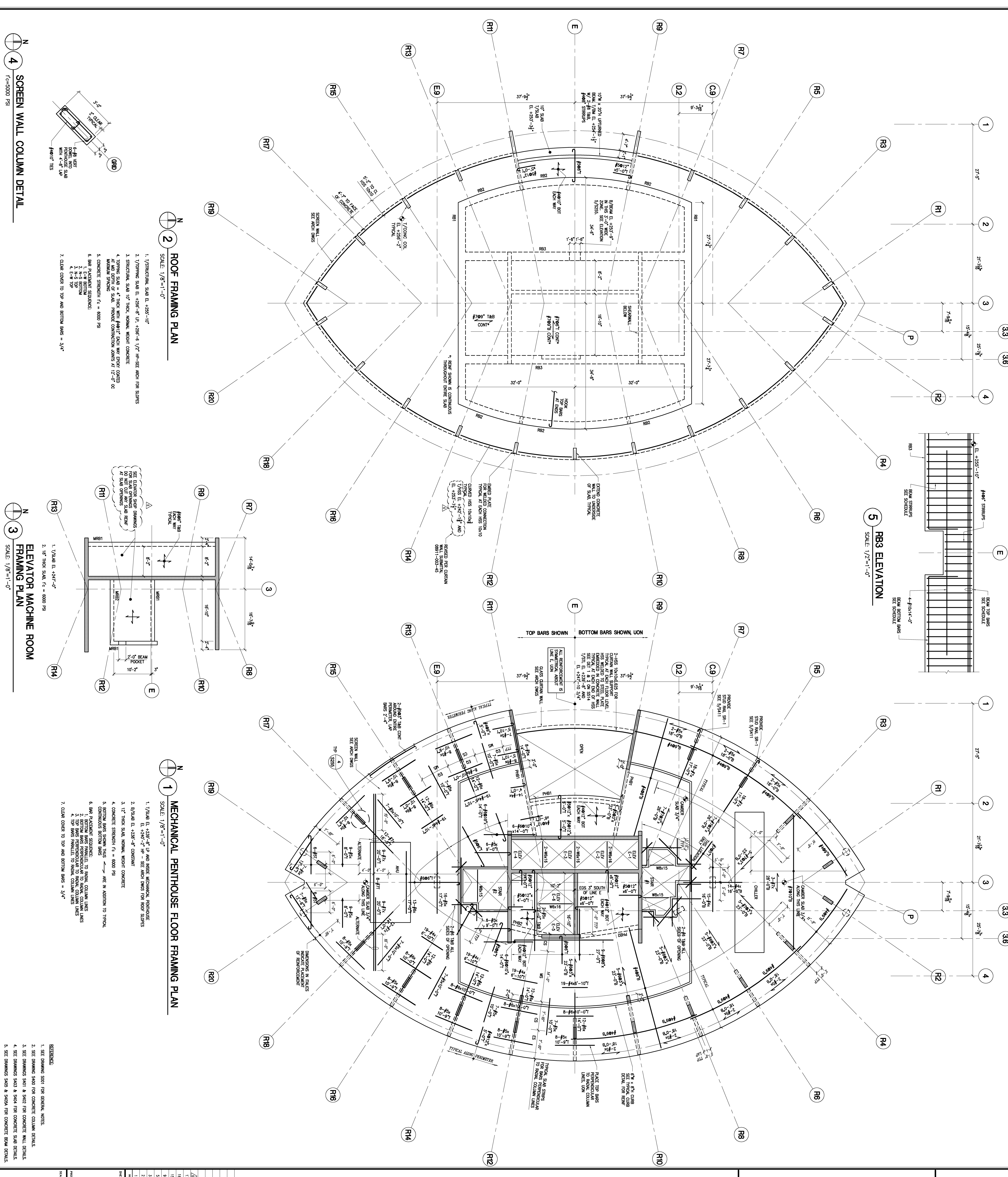
MECHANICAL PENTHOUSE AND ROOF FRAMING PLAN

PROJECT: MECHANICAL PENTHOUSE AND ROOF FRAMING PLAN
 DRAWN BY: [Name]
 CHECKED BY: [Name]
 PROJECT NO.: CCM08700

MECHANICAL PENTHOUSE AND ROOF FRAMING PLAN

REVISIONS:

- SET DRAWING 5011 FOR GENERAL NOTES
- SET DRAWING 5040 FOR CONCRETE COLUMN DETAILS
- SET DRAWING 5041 & 5042 FOR CONCRETE WALL DETAILS
- SET DRAWING 5043 & 5044 FOR CONCRETE SLAB DETAILS
- SET DRAWING 5045 & 5046 FOR CONCRETE BEAM DETAILS



4 SCREEN WALL COLUMN DETAIL
SCALE: 1/2"=1'-0"

2 ROOF FRAMING PLAN
SCALE: 1/8"=1'-0"

3 RB3 ELEVATION
SCALE: 1/2"=1'-0"

4 ELEVATOR MACHINE ROOM FRAMING PLAN
SCALE: 1/8"=1'-0"

1 MECHANICAL PENTHOUSE FLOOR FRAMING PLAN
SCALE: 1/8"=1'-0"



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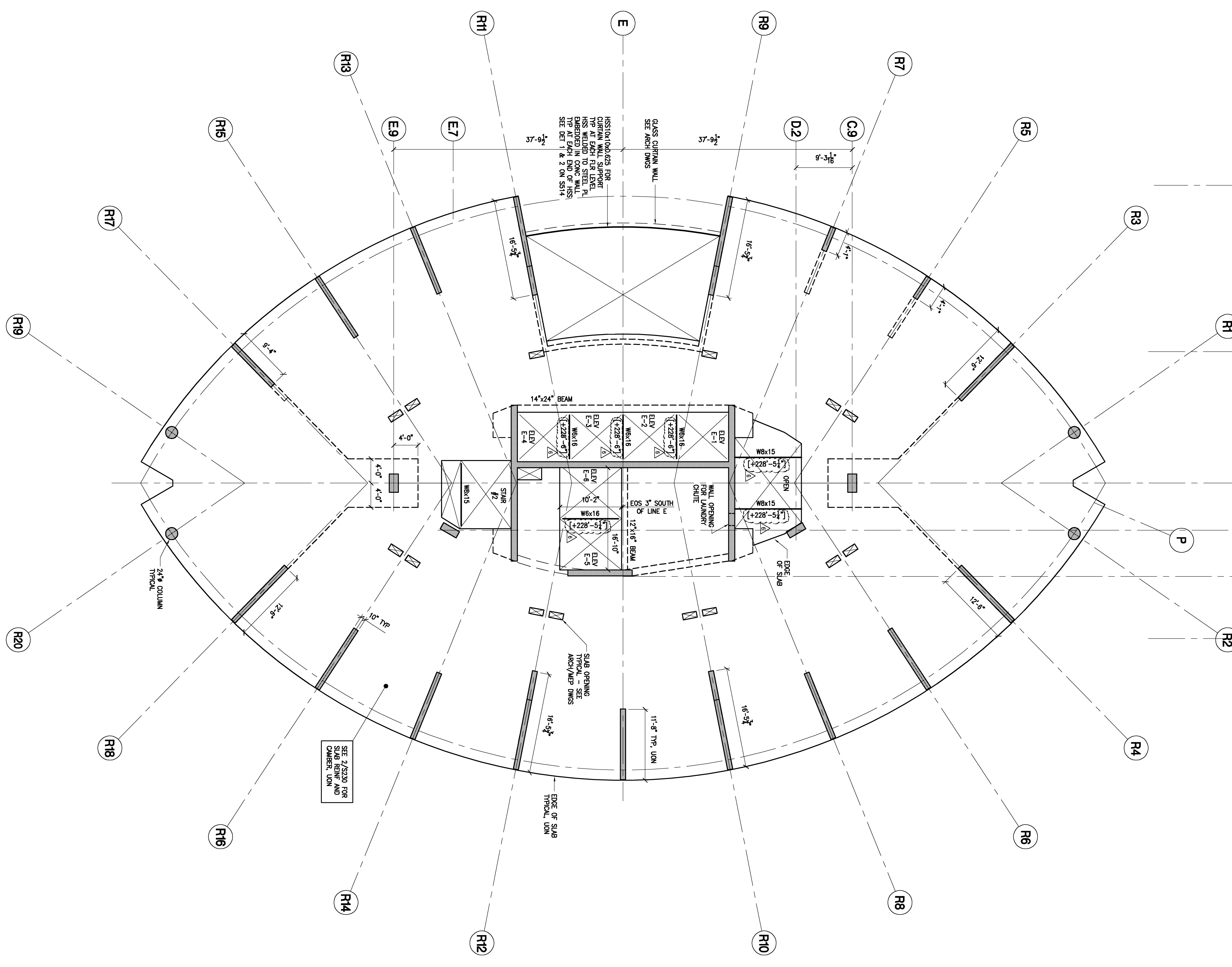
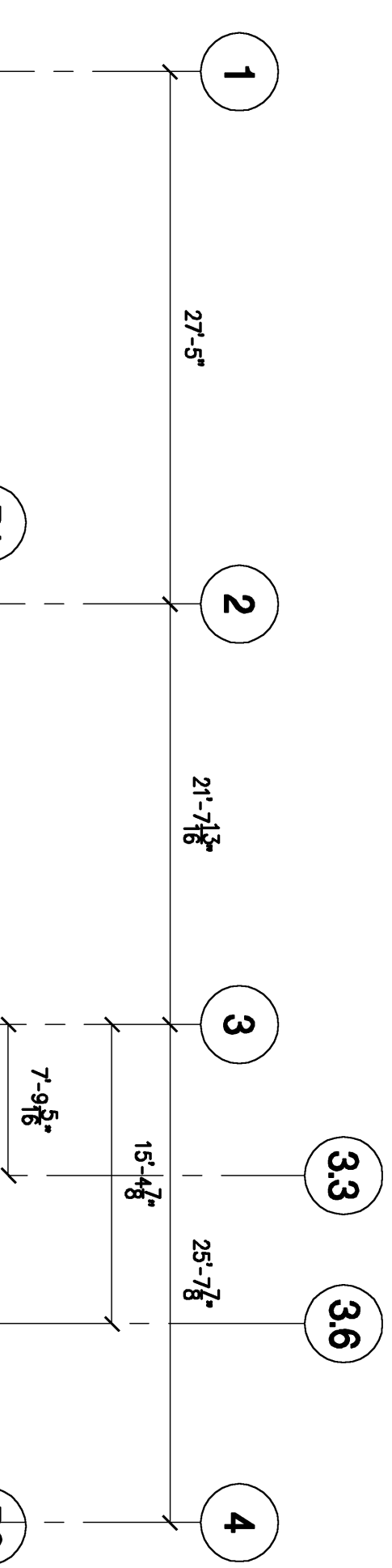
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23RD FLOOR FRAMING PLAN
SCALE: 1/8"=1'-0"

1. 7/8" DIA. 4228-8"
2. 7/8" THICK SLAB, 10% NORMAL WEIGHT CONCRETE
3. CONCRETE STRENGTH (f'c) = 5000 PSI AT 28 DAYS - 8TH FLOOR
4. BOTTOM BARS SPACING THIS " " ARE IN ACCORD TO TYPICAL CONTINUOUS BOTTOM BARS
5. BARS PLACEMENT SEQUENCE:
1. BOTTOM BARS PARALLEL TO ROWS COLUMN LINES
2. BOTTOM BARS PARALLEL TO COLUMNS LINES
3. TOP BARS PARALLEL TO ROWS COLUMN LINES
4. TOP BARS PARALLEL TO COLUMNS LINES
6. CLEAR COVER TO TOP AND BOTTOM BARS = 3/4"

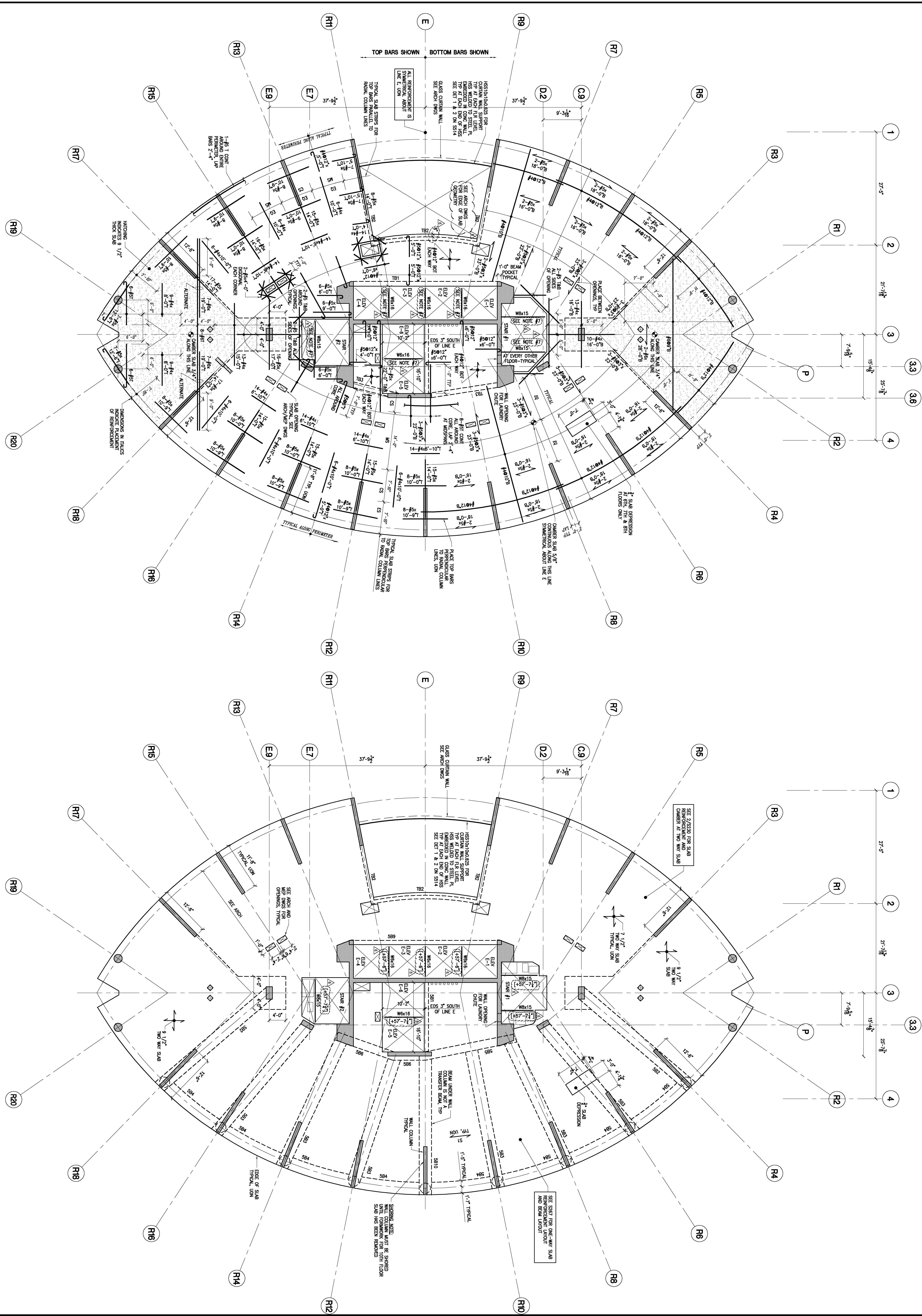
- REFERENCE:**
1. SEE DRAWING S001 FOR GENERAL NOTES
 2. SEE DRAWING S400 FOR CONCRETE COLUMN DETAILS
 3. SEE DRAWING S401 & S402 FOR CONCRETE WALL DETAILS
 4. SEE DRAWING S403 & S404 FOR CONCRETE SLAB DETAILS
 5. SEE DRAWING S405 & S406 FOR CONCRETE BEAM DETAILS

NO.	DATE	DESCRIPTION
1	29 JUN 05	ADDITION #1
2	02 DEC 05	ISSUED FOR CONSTRUCTION AND PERMIT
3	24 OCT 05	OWNER AND MARRIOTT CD REVIEW
4	20 MAY 05	ISSUED CONCRETE BID PACKAGE
5	29 JUN 05	REVISED DESIGN DEVELOPMENT
6	12 FEB 05	ISSUED DESIGN DEVELOPMENT
7	12 FEB 05	ISSUED DESIGN DEVELOPMENT

23RD FLOOR FRAMING PLAN

PROJECT LOCATION	23rd & Grand Rapids
SCALE	AS SHOWN
DRAWN BY	AS SHOWN
CHECKED BY	CCM/STW
PROJECT NO.	CCM/STW
DATE PLOTTED	05/24/05

S240



6TH-2ND FLOOR FRAMING PLAN
SCALE: 1/8"=1'-0"

5TH FLOOR FRAMING PLAN
SCALE: 1/8"=1'-0"

1. 1/2" SLAB E.L. SEE LIST FOR THIS DRAWING
2. 7" 1/2" THICK SLAB, UNK. NORMAL WEIGHT CONCRETE
3. CONCRETE STRENGTH, $f_c = 4000$ PSI, TYPICAL
4. BOTTOM BARS SHOWN THIS WAY ARE IN ADDITION TO TYPICAL CONTINUOUS BOTTOM BARS
5. BAR PLACEMENT SEQUENCE:
 1. TOP BARS PERPENDICULAR TO RADIAL COLUMN LINES
 2. BOTTOM BARS PERPENDICULAR TO RADIAL COLUMN LINES
 3. TOP BARS PARALLEL TO RADIAL COLUMN LINES
 4. BOTTOM BARS PARALLEL TO RADIAL COLUMN LINES
6. CLEAR COVER TO TOP AND BOTTOM BARS = 3/4"
7. TYPICAL E.L. OF BEAMS: E1-E4 = 2" BELOW 1/2" SLAB E.L.; E5-E9 = 4" BELOW 1/2" SLAB E.L.

FLOOR ELEVATIONS

6TH FLOOR	+487'-2"
5TH FLOOR	+486'-0"
4TH FLOOR	+484'-8"
3RD FLOOR	+483'-6"
2ND FLOOR	+482'-4"
1ST FLOOR	+481'-2"
0TH FLOOR	+480'-0"
-1ST FLOOR	+478'-8"
-2ND FLOOR	+477'-6"
-3RD FLOOR	+476'-4"
-4TH FLOOR	+475'-2"
-5TH FLOOR	+474'-0"
-6TH FLOOR	+472'-8"
-7TH FLOOR	+471'-6"
-8TH FLOOR	+470'-4"
-9TH FLOOR	+469'-2"
-10TH FLOOR	+468'-0"
-11TH FLOOR	+466'-8"
-12TH FLOOR	+465'-6"
-13TH FLOOR	+464'-4"
-14TH FLOOR	+463'-2"
-15TH FLOOR	+462'-0"
-16TH FLOOR	+460'-8"
-17TH FLOOR	+459'-6"
-18TH FLOOR	+458'-4"
-19TH FLOOR	+457'-2"
-20TH FLOOR	+456'-0"
-21ST FLOOR	+454'-8"
-22ND FLOOR	+453'-6"
-23RD FLOOR	+452'-4"
-24TH FLOOR	+451'-2"
-25TH FLOOR	+450'-0"

- REFERENCES:**
1. SEE DRAWING S001 FOR GENERAL NOTES
 2. SEE DRAWING S002 FOR CONCRETE COLUMN DETAILS
 3. SEE DRAWING S001 & S002 FOR CONCRETE WALL DETAILS
 4. SEE DRAWING S003 & S004 FOR CONCRETE SLAB DETAILS
 5. SEE DRAWING S005 & S006 FOR CONCRETE BEAM DETAILS

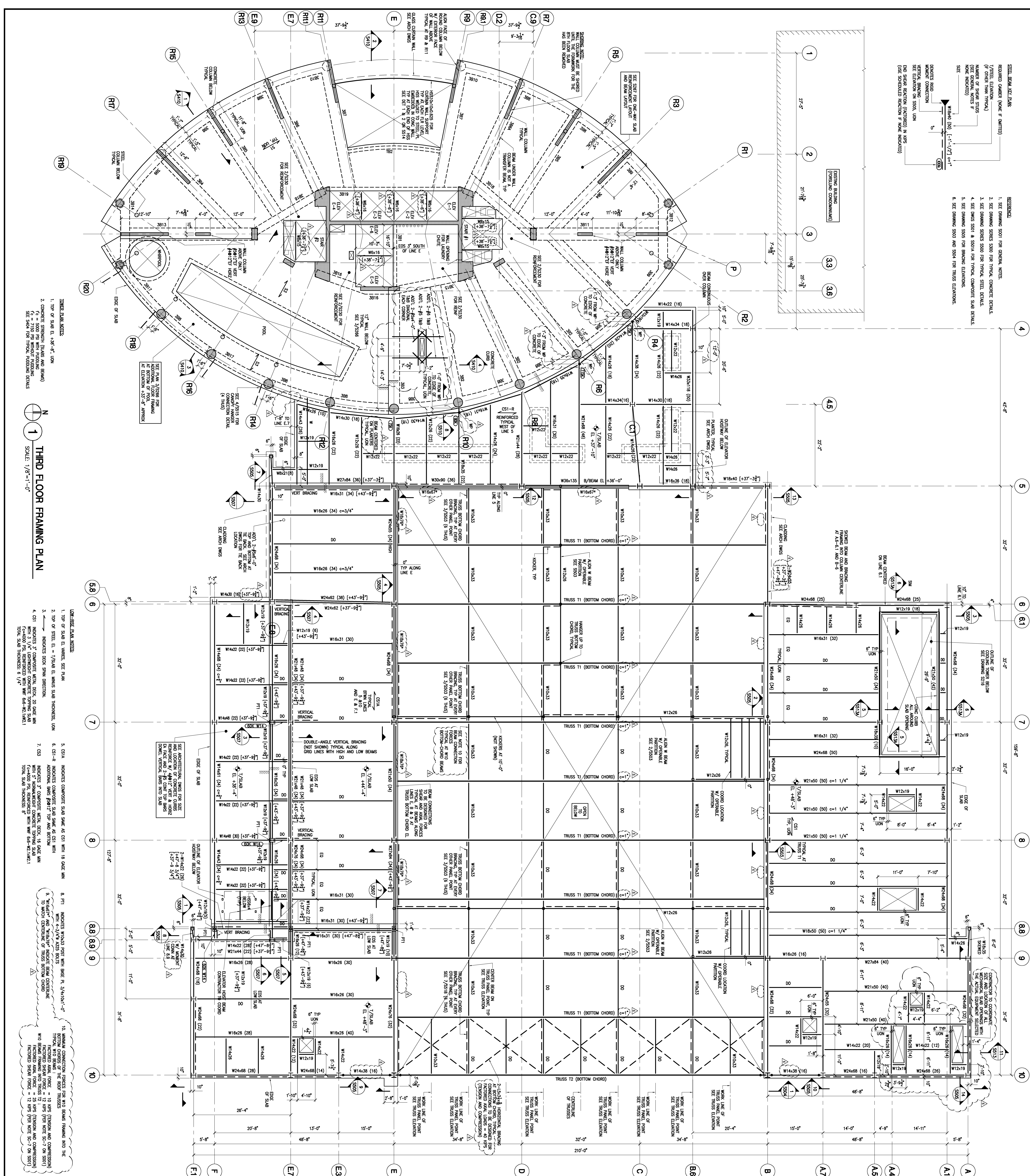
- REFERENCES:**
1. SEE DRAWING S001 FOR GENERAL NOTES
 2. SEE DRAWING S002 FOR CONCRETE COLUMN DETAILS
 3. SEE DRAWING S001 & S002 FOR CONCRETE WALL DETAILS
 4. SEE DRAWING S003 & S004 FOR CONCRETE SLAB DETAILS
 5. SEE DRAWING S005 & S006 FOR CONCRETE BEAM DETAILS

5TH AND 6TH-2ND FLOOR FRAMING PLANS

NO.	DATE	DESCRIPTION
1	09 JAN 06	ADDITION #1
2	02 DEC 05	ISSUED FOR CONSTRUCTION AND PERMIT
3	28 OCT 05	OWNER AND ARCHITECT CD REVIEW
4	24 OCT 05	OWNER AND ARCHITECT CD REVIEW
5	20 MAR 05	ISSUED FOR CONSTRUCTION
6	29 APR 05	ISSUED FOR CONSTRUCTION
7	15 APR 05	ISSUED FOR CONSTRUCTION
8	10 APR 05	ISSUED FOR CONSTRUCTION
9	07 APR 05	ISSUED FOR CONSTRUCTION

PROJECT LOCATION: 2201 LYON ST NW, SUITE 405 • GRAND RAPIDS, MI 49503
 PROJECT NO.: S230
 DRAWN BY: AS SHOWN
 CHECKED BY: CCM/STW
 PROJECT NO.: S230

REQUIRED CARRIER (NAME IF OMITTED)
 1. SET DRAWING SECTIONS 5/8" FOR TYPICAL CONCRETE DETAILS.
 2. SET DRAWING SECTIONS 5/8" FOR TYPICAL STEEL DETAILS.
 3. SET DRAWING SECTIONS 5/8" FOR TYPICAL CONCRETE SLAB DETAILS.
 4. SET DRAWING SECTIONS 5/8" FOR BRIDGE ELEVATIONS.
 5. SET DRAWING SECTIONS 5/8" FOR TRUSS ELEVATIONS.
 6. SET DRAWING SECTIONS 5/8" FOR MASS ELEVATIONS.



THIRD FLOOR FRAMING PLAN
 SCALE: 1/8"=1'-0"
 THIRD FLOOR FRAMING PLAN

LOW-RISE PLAN NOTES:
 1. TOP OF SLAB EL. = 38'-4" ON
 2. TOP OF STEEL EL. = 37'-0" ON
 3. INDICATES DECK SPAN DIRECTION
 4. CSI INDICATES 3" COMPACT METAL DECK, 18 GAUGE, 1/4" THICK, REINFORCED WITH W1W 60-26 (W12X11) TOTAL SLAB THICKNESS: 6 1/4"

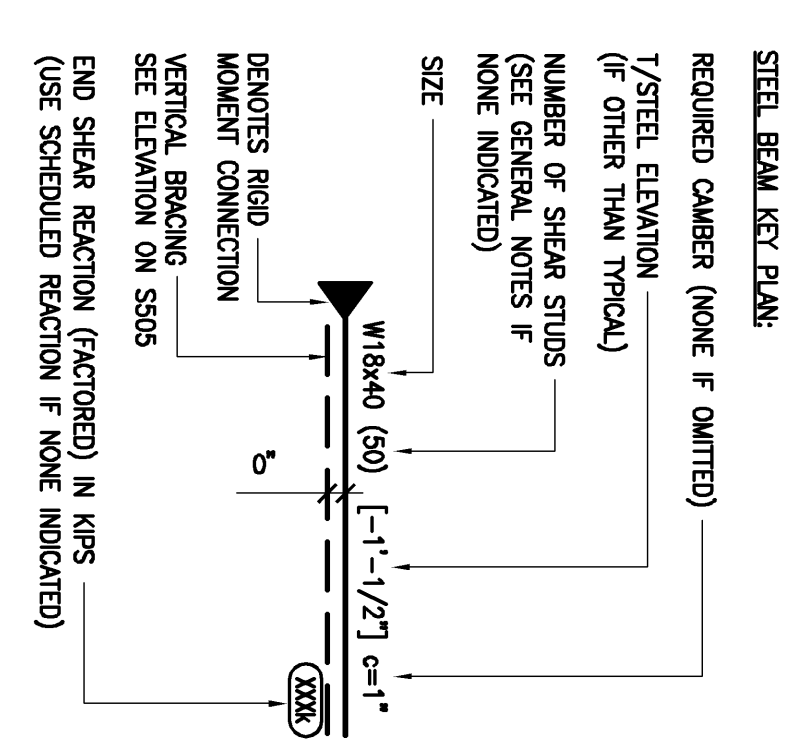
5. CSI A INDICATES COMPOSITE SLAB SAME AS CSI WITH 18 GAUGE LUM
 6. CSI-B INDICATES COMPOSITE SLAB SAME AS CSI WITH 18 GAUGE LUM
 7. CSI-C INDICATES 3" COMPACT METAL DECK, 18 GAUGE, 1/4" THICK, REINFORCED WITH W1W 60-26 (W12X11) TOTAL SLAB THICKNESS: 6 1/4"
 8. PT1 INDICATES W10X30 POST WITH BOLT IN 3/4" x 10" x 10" WITH 4-3/4" ANCHOR BOLTS
 9. "TRUSS" AND "W12X22" INDICATE BEAM CENTERLINE TO WHICH CENTERLINE OF TRUSS BOTTOM CHORD
 10. MINIMAL CONNECTION DRESS FOR W10 BEAMS FRAME INTO THE BOTTOM CHORDS OF THE TRUSS
 11. DIMENSIONS IN PARETHESIS ARE 10 INCHES (TRUSS AND COMPRESSION) AND 12 INCHES (TENSION) UNLESS NOTED OTHERWISE. DIMENSIONS IN PARETHESIS ARE 12 INCHES (TENSION AND COMPRESSION) UNLESS NOTED OTHERWISE.

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FSHBERGER CONSULTING ENGINEERS ARCHITECTS 1515 Kalamazoo Drive, SE Grand Rapids, Michigan 49508 Tel: 616-933-3333 www.fshberger.com		DANIEL WEINRACH & PARTNERS, LTD. Structural Engineer 831 W. 1st St. SE Grand Rapids, MI 49503 Tel: 616-312-6277 www.danielweinrach.com	

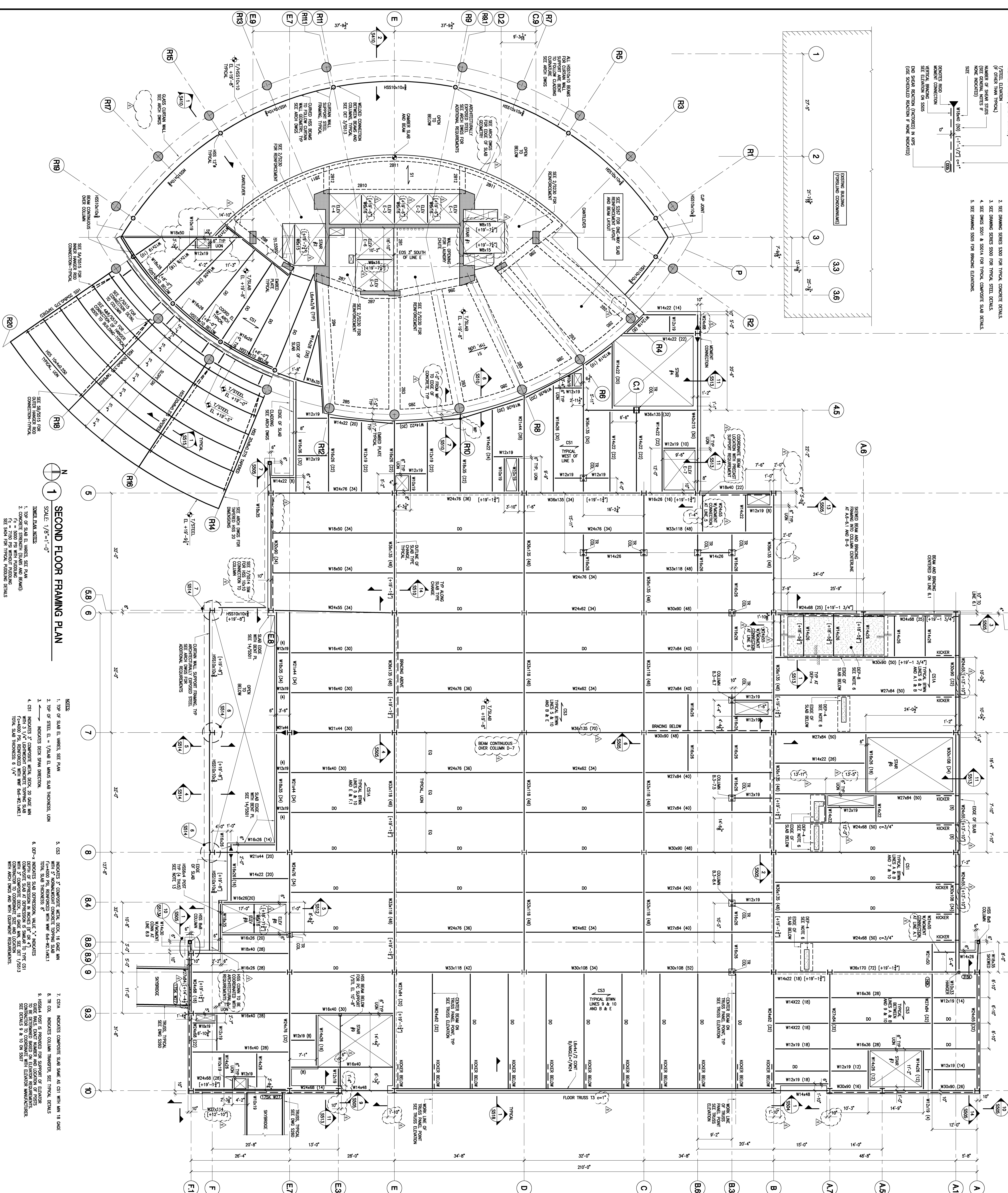
NO.	DATE	DESCRIPTION
1	09 JUN 06	ADDENDUM #1
2	12 DEC 05	ISSUED FOR CONSTRUCTION AND PERMIT
3	28 OCT 05	OWNER AND ARCHITECT CD REVIEW
4	24 OCT 05	OWNER AND ARCHITECT CD REVIEW
5	23 SEP 05	NO. ARCHITECT #1 - STRUCTURAL STEEL & FOUNDATIONS
6	09 AUG 05	STRUCTURAL STEEL & FOUNDATIONS
7	28 JUN 05	REVISION DESIGN DEVELOPMENT
8	19 APR 05	REVISION DESIGN DEVELOPMENT
9	10 DEC 04	CONSTRUCTION DESIGN

PROJECT: THIRD FLOOR FRAMING PLAN
 SCALE: AS SHOWN
 SHEET NO.: S221

1
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33
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84
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89
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1. SET DRAWING SERIES 5300 FOR TYPICAL CONCRETE DETAILS.
 2. SET DRAWING SERIES 5300 FOR TYPICAL STEEL DETAILS.
 3. SET DRAWING SERIES 5300 FOR TYPICAL CONCRETE SLAB DETAILS.
 4. SET DRAWING SERIES 5300 FOR TYPICAL CONCRETE SLAB DETAILS.
 5. SET DRAWING SERIES 5300 FOR BRACING ELEVATIONS.



SECOND FLOOR FRAMING PLAN
 SCALE: 1/8"=1'-0"
 NOTES:
 1. TOP OF SLAB EL. WARS. SEE PLAN.
 2. TOP OF STEEL EL. = 7'-6" AB. WARS. SLAB THICKNESS. UN.
 3. INDICATES BEAM SPAN DIRECTION.
 4. CSI INDICATES 3" CONCRETE MESH. 20 GA. MIN. (400) P.S. REINFORCED WITH WWS #6 @ 12" ON CENTER. TOTAL SLAB THICKNESS IS 1'-4".
 5. CSI INDICATES 3" CONCRETE MESH. 16 GA. MIN. WITH 5" NOMINATED CONCRETE TOPPING SLAB WITH 3" NOMINATED CONCRETE TOPPING SLAB. TOTAL SLAB THICKNESS IS 2'-0" WITH WWS #6 @ 12" ON CENTER.
 6. CSI INDICATES 3" CONCRETE MESH. 20 GA. MIN. (400) P.S. REINFORCED WITH WWS #6 @ 12" ON CENTER. TOTAL SLAB THICKNESS IS 1'-4".
 7. CSI14 INDICATES CONCRETE SLAB WITH 16 GA. WWS WITH 5" NOMINATED CONCRETE TOPPING SLAB. TOTAL SLAB THICKNESS IS 2'-0" WITH WWS #6 @ 12" ON CENTER.
 8. TR. COL. INDICATES COLUMN TRUSSER. SEE TYPICAL DETAILS.
 9. RESISTOR PANELS. EXACT NUMBER AND LOCATION OF RESISTOR PANELS TO BE DETERMINED BASED ON ELEVATION REQUIREMENTS. SEE DETAILS 9 & 10 ON S027.
 10. TRUSS TYPICAL SYMBOLOGY.

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PROJECT INFORMATION
 PROJECT: SECOND FLOOR FRAMING PLAN
 SCALE: AS SHOWN
 DRAWN BY: [Name]
 CHECKED BY: [Name]
 PROJECT NO.: [Number]
 SHEET NO.: S211



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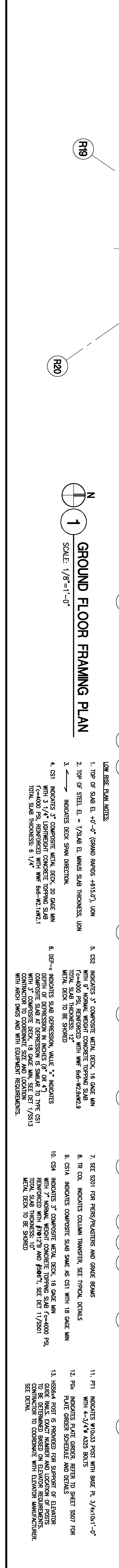
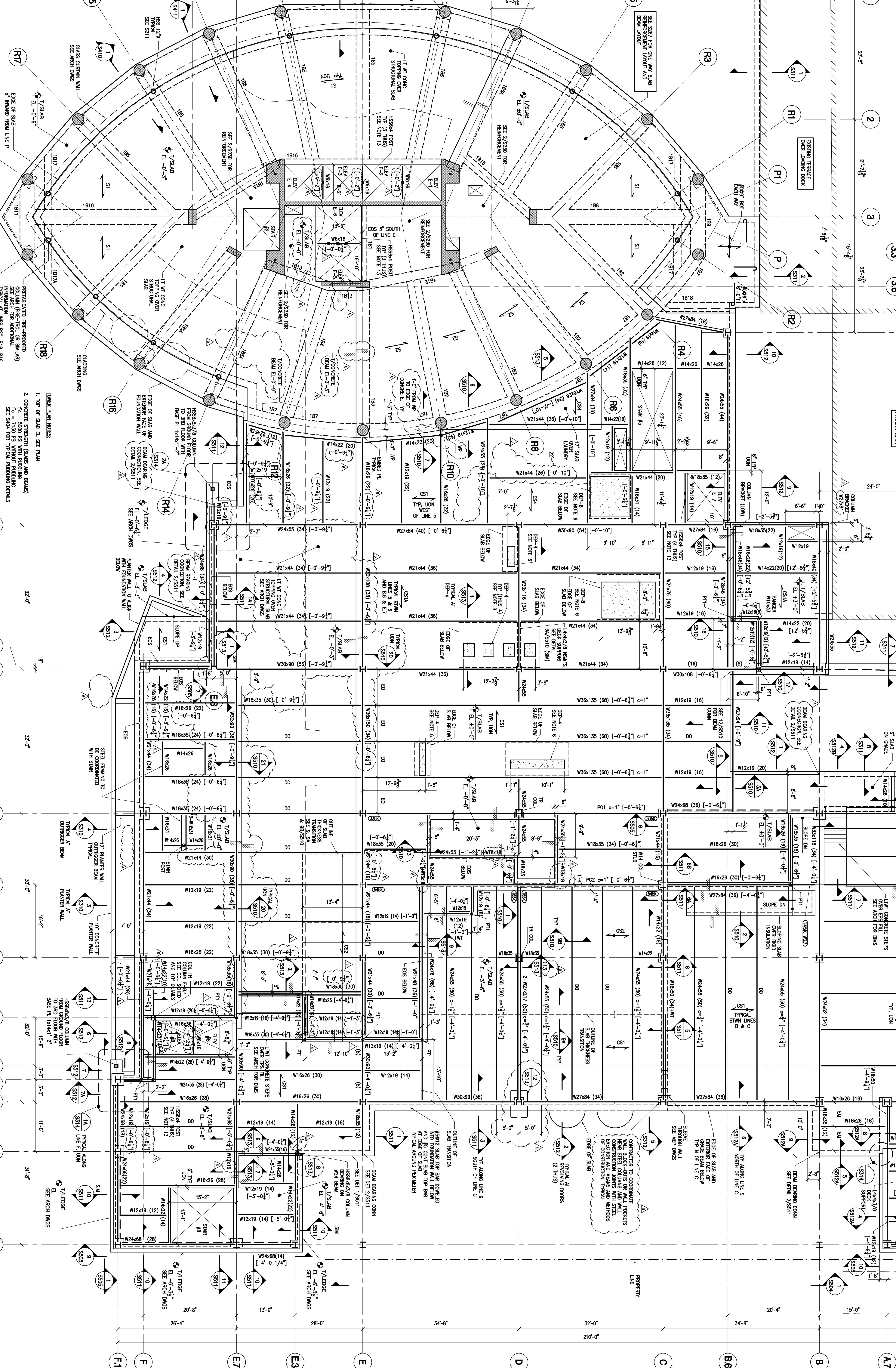
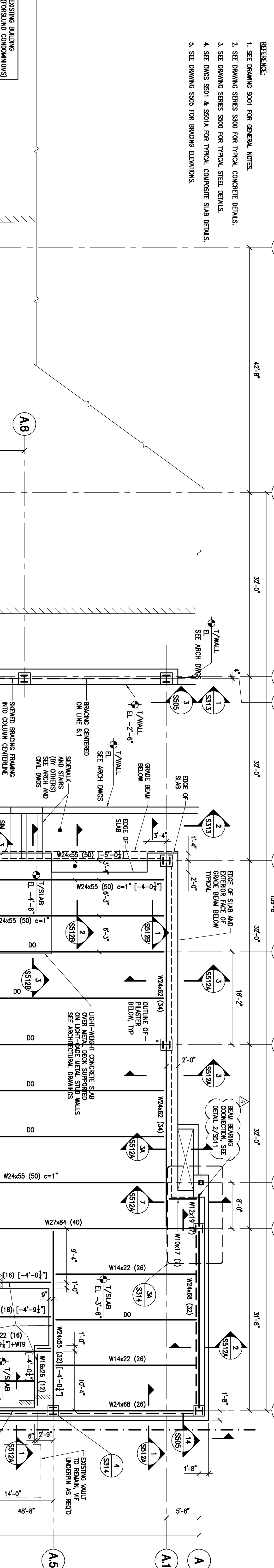
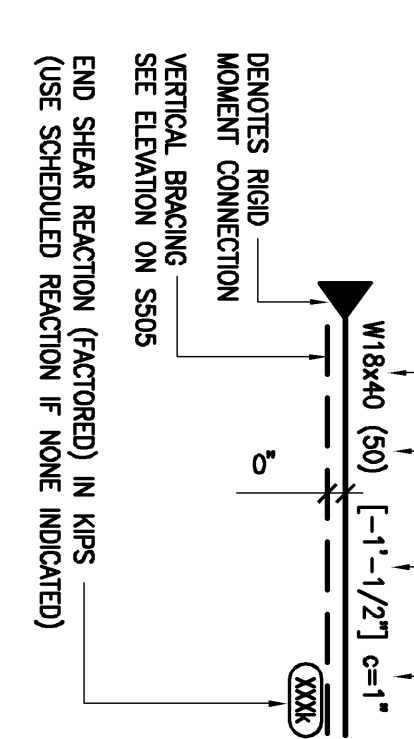
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- REQUIRED CANNOT (NAME IF OMITTED)
1. SET DRAWING SERIES 500 FOR GENERAL NOTES.
 2. SET DRAWING SERIES 500 FOR TYPICAL CONCRETE DETAILS.
 3. SET DRAWING SERIES 500 FOR TYPICAL STEEL DETAILS.
 4. SET DWGS 501 & 501A FOR TYPICAL CONCRETE SLAB DETAILS.
 5. SET DRAWING 500 FOR BRACING ELEVATIONS.



NO.	REVISION	DATE	BY	CHKD.
1	ISSUED FOR CONSTRUCTION AND PERMIT	08/14/10	AS	AS
2	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
3	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
4	ISSUED FOR CONSTRUCTION AND PERMIT	08/14/10	AS	AS
5	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
6	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
7	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
8	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
9	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
10	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
11	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
12	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
13	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
14	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
15	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
16	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
17	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
18	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
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20	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
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22	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
23	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
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25	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
26	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
27	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
28	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
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49	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS
50	OWNER AND MARRIOTT CD REVIEW	08/14/10	AS	AS

AS SHOWN
CC-0470
S206

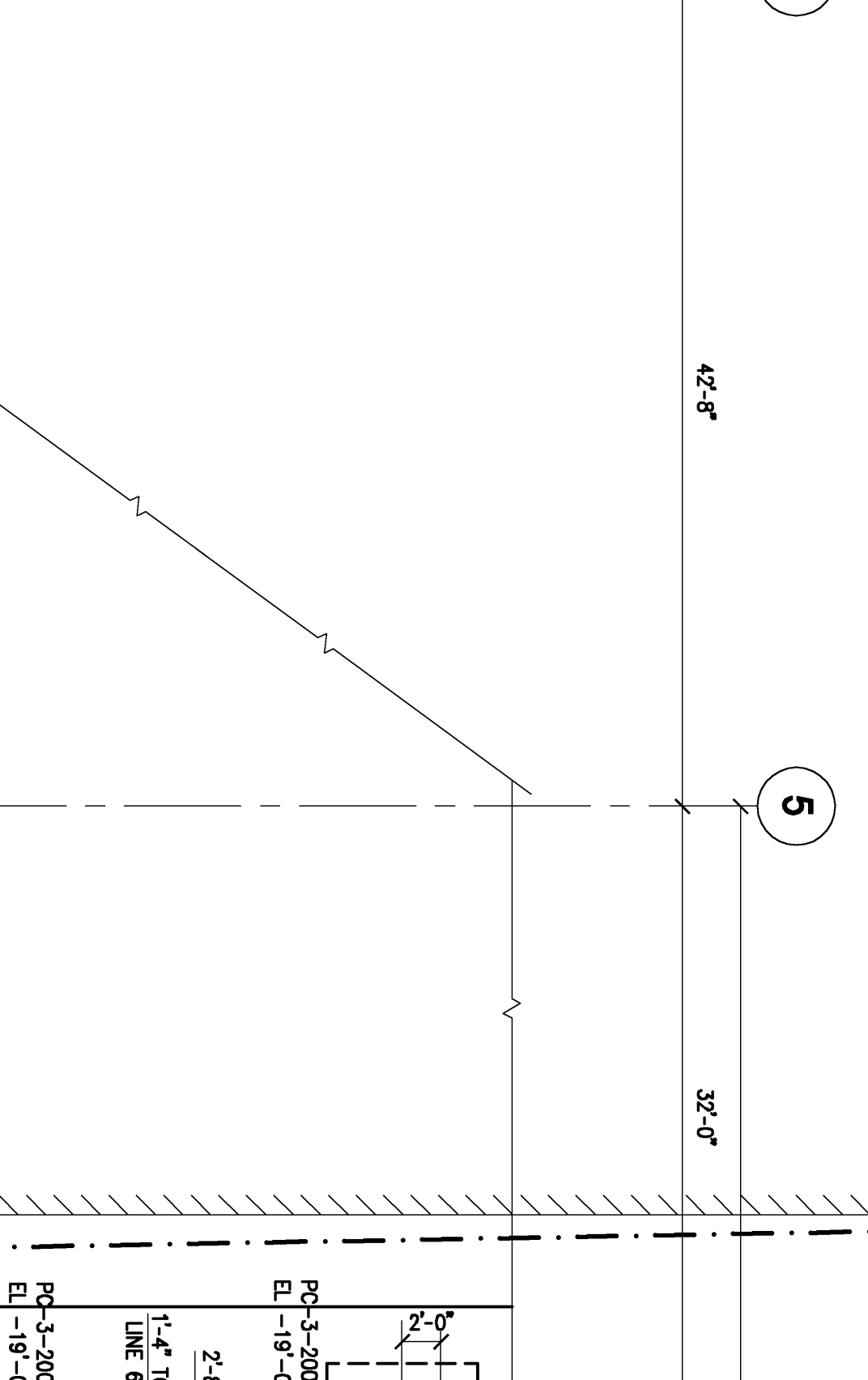
SECTION FOR WALLS, FLOOR AND ROOF

1. SEE DRAWING S001 FOR GENERAL NOTES.

2. SEE DRAWING SERIES S000 FOR TYPICAL CONCRETE DETAILS.

3. SEE DRAWING SERIES S000 FOR TYPICAL FOUNDATION DETAILS.

4. SEE DRAWING SERIES S000 FOR TYPICAL STEEL DETAILS.



GENERAL NOTES:

1. SEE DRAWING S001 FOR GENERAL NOTES.

2. SEE DRAWING SERIES S000 FOR TYPICAL CONCRETE DETAILS.

3. SEE DRAWING SERIES S000 FOR TYPICAL FOUNDATION DETAILS.

4. SEE DRAWING SERIES S000 FOR TYPICAL STEEL DETAILS.

5. SEE DRAWING S001 FOR GENERAL NOTES.

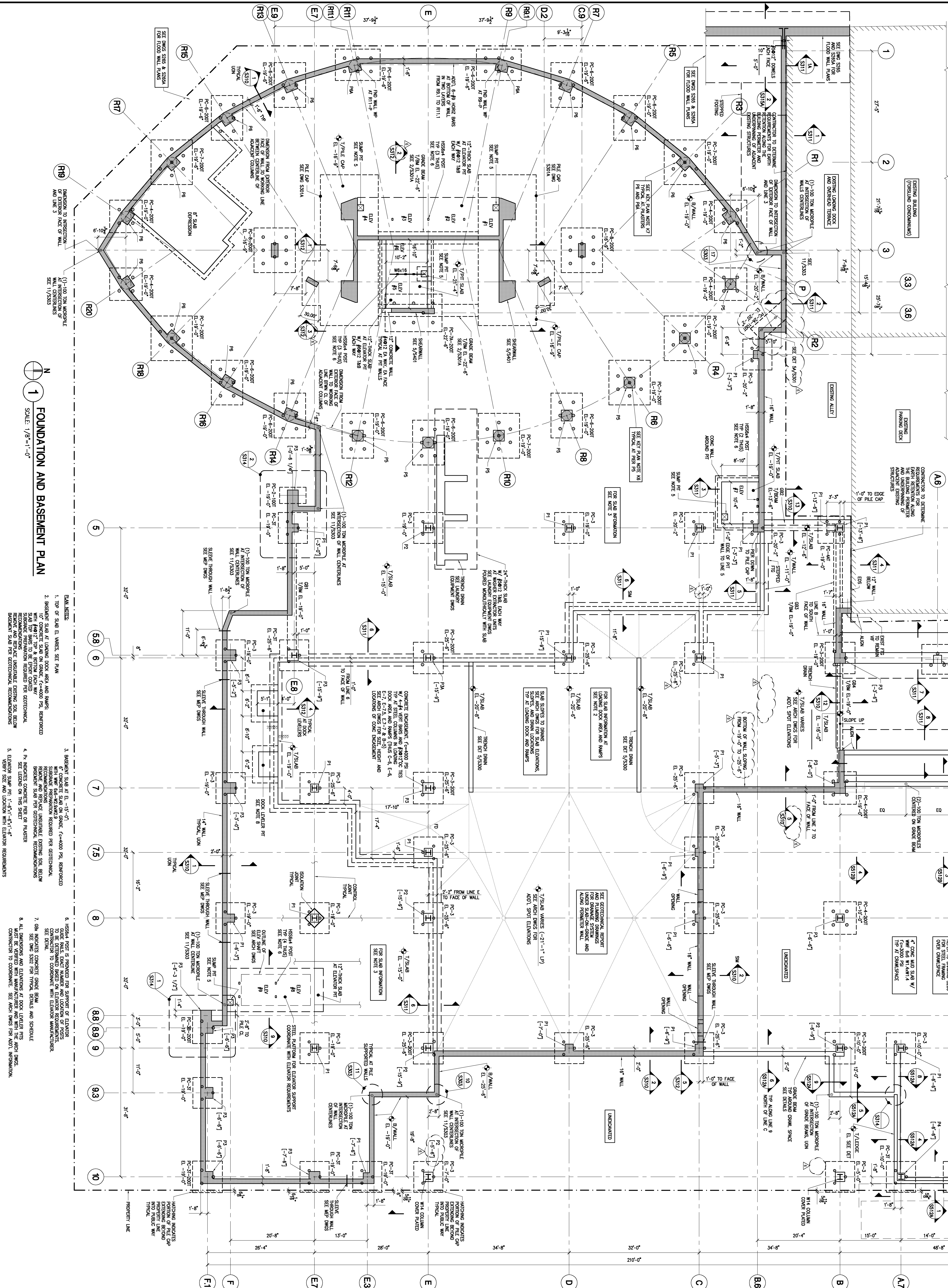
6. SEE DRAWING S001 FOR GENERAL NOTES.

7. SEE DRAWING S001 FOR GENERAL NOTES.

8. SEE DRAWING S001 FOR GENERAL NOTES.

9. SEE DRAWING S001 FOR GENERAL NOTES.

10. SEE DRAWING S001 FOR GENERAL NOTES.



FOUNDATION AND BASEMENT PLAN

SCALE: 1/8"=1'-0"

1. TOP OF SLAB IS SHOWN. SEE PLAN.
2. BASEMENT SLAB AT LANDING DOCK AREA AND RAUMS. SEE PLAN.
3. BASEMENT SLAB AT EL. -15'-0" (SEE PLAN). SEE PLAN.
4. SEE PLAN FOR TYPICAL DETAILS AND SCHEDULE.
5. ELEVATOR SHAFT AT EL. -15'-0" TO -11'-0". SEE PLAN.
6. SEE PLAN FOR TYPICAL DETAILS AND SCHEDULE.
7. SEE PLAN FOR TYPICAL DETAILS AND SCHEDULE.
8. SEE PLAN FOR TYPICAL DETAILS AND SCHEDULE.
9. SEE PLAN FOR TYPICAL DETAILS AND SCHEDULE.
10. SEE PLAN FOR TYPICAL DETAILS AND SCHEDULE.

NO.	DESCRIPTION
1	FOUNDATION AND BASEMENT PLAN
2	FOUNDATION AND BASEMENT PLAN
3	FOUNDATION AND BASEMENT PLAN
4	FOUNDATION AND BASEMENT PLAN
5	FOUNDATION AND BASEMENT PLAN
6	FOUNDATION AND BASEMENT PLAN
7	FOUNDATION AND BASEMENT PLAN
8	FOUNDATION AND BASEMENT PLAN
9	FOUNDATION AND BASEMENT PLAN
10	FOUNDATION AND BASEMENT PLAN

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APPENDIX B: HAND CALCULATIONS



ALTICOR

CBEAM

GROUND - 12

$$V_u = 213 \text{ k}$$

$$M_u = 213 \left(\frac{8}{2} \right) = 852 \text{ kft}$$

$$A_s = \frac{852}{4.3(27)} = 7.33 \text{ in}^2 \quad 4\#11 = 6.24 \text{ in}^2$$

$$M_{pr} = \frac{6.24(4.3)(27)}{0.9} \times 1.25 = 1006 \text{ kft}$$

$$V_n = \frac{2M_{pr}}{\ell} = \frac{2}{8}(1006) = \underline{251 \text{ k}} > 213 \text{ k}$$

$$\phi V_s = \phi A_v f_y d/s \rightarrow 0.75(60) \frac{A_v}{12''} \times 27 > 251$$

$$A_v/\text{ft} = \frac{251(12)}{27(0.75)(60)} = 2.48 \text{ in}^2/\text{ft}$$

TRY 4 LEGS #5 @ 6"

$$4(.31) \left(\frac{12''}{6''} \right) = 2.48 \text{ in}^2/\text{ft}$$

$$251 < \phi 8 \sqrt{f'_c} b d$$

$$251 = \phi \underline{5.44} \sqrt{f'_c} b d$$

∴ USE 4#11 TOP & BOTTOM

W/ 4 VERTICAL LEGS #5 @ 6 IN O.C.

C BEAM

FLOOR 13-18

$$V_{ETABS} = 225^k$$

$$V_u = 182^k \quad V_s = 180^k$$

$$M_u = 182^k \times 8 \times \frac{1}{2} = 728 \text{ kFT}$$

$$A_s = \frac{M_u}{c \cdot d} = \frac{728}{4.3(27)} = 6.27 \text{ in}^2 \quad 4\#10 = 5.08 \text{ in}^2$$

$$M_{pr} = \frac{A_s (c) d}{0.9} \times 1.25 = \frac{5.08(4.3)(27)}{0.9} \times 1.25 = 819 \text{ kFT}$$

$$V_n = \frac{M_{pr} \times 2}{l} = \frac{819 \left(\frac{2}{8}\right)}{8} = \underline{205^k} > 181^k$$

$$\phi V_s = \phi A_v f_y \frac{d}{s} = 0.75(4 \text{ LEGS})(0.31)(60)\left(\frac{27}{6}\right) = 251^k$$

$$\phi V_s = 251 \left(\frac{6}{8}\right) = 188^k$$

⇒ MUST USE 6" SPACING

$$251^k < \phi 8 \sqrt{f'_c} b d$$

$$251 = \phi \underline{5.44} \sqrt{f'_c} b d$$

$$4\#4? \phi V_s = 251 \left(\frac{.20}{.31}\right) = 162^k$$

⇒ 4#4 NO GOOD

∴ USE 4#10 TOP & BOTTOM

W/ 4 LEGS #5 @ 6 IN O.C.

* METHOD ABOVE IS A SHORT CUT, BELOW IS LONG WAY

$$a = \frac{A_s f_y}{0.85 f'_c b} = \frac{5.08(60)}{0.85(9)(24)} \times 1.25 = 2.08 \text{ in}$$

$$\phi M_n = 5.08 \text{ in} (1.25 \times 60) \left(27 - \frac{2.08}{2}\right) \times \frac{1}{2} = 824 \text{ kFT} \approx 819$$

$$* \frac{M_u}{c d} = 5.08 \rightarrow c = \frac{824}{27(5.08)} \times \frac{0.9}{1.25} = 4.3$$

C BEAM

19 - ROOF

$$V_u = 122 \text{ k}$$

$$M_u = 122 \left(\frac{8}{2} \right) = 488 \text{ k-ft}$$

$$A_s = \frac{488}{4.3(27)} \times 1.25 = 5.25 \text{ in}^2 \quad \text{CHECK } 4\#9 \quad A_s = 4 \text{ in}^2$$

$$a = \frac{4(60)}{0.85(9)(24)} = 1.31 \text{ in}$$

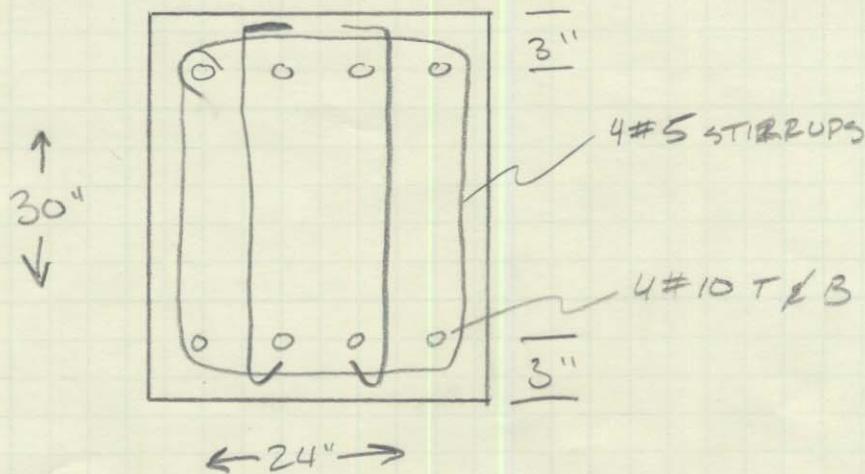
$$M_{pr} = A_s f_y \left(d - \frac{a}{2} \right) = 4(1.25 \times 60) \left(27 - \frac{1.31}{2} \right) \times \frac{1}{12} = 442 \text{ k-ft}$$

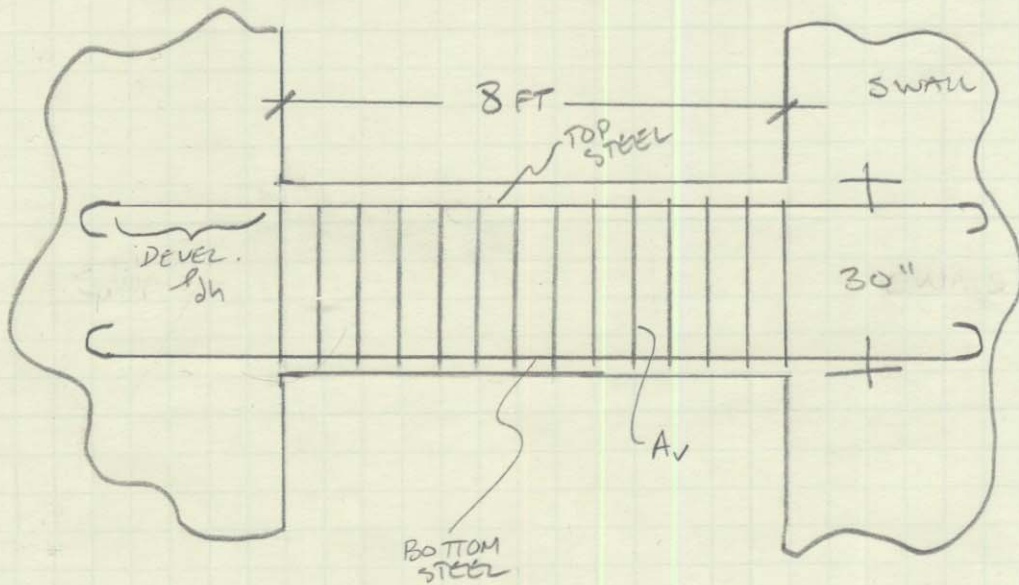
$$V_u = 442 \left(\frac{2}{8'} \right) = 111 \text{ k} \quad \underline{\text{NO GOOD}}$$

USE 4#10 $A_s = 5.08 \text{ in}^2$ FOR EASE OF CONSTRUCTION
W/ 4#5 VERTICAL

$$\left. \begin{array}{l} V_u = 205 \text{ k} \\ \phi V_u = 251 \text{ k} \end{array} \right\} \text{SEE CBEAM FLOOR 13-18}$$

EXAMPLE DETAIL





l_{dh} : MUST USE 180° HOOK IN HIGH SEISMICITY

$$l_{dh} = \frac{f_y \alpha \beta \lambda}{20 \sqrt{f'_c}} d_b$$

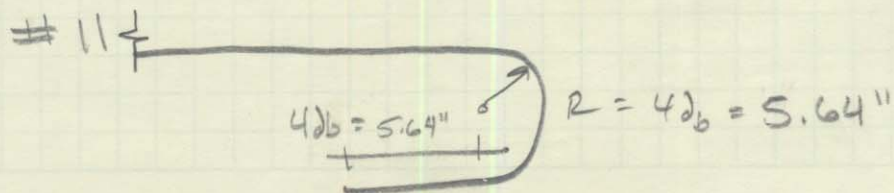
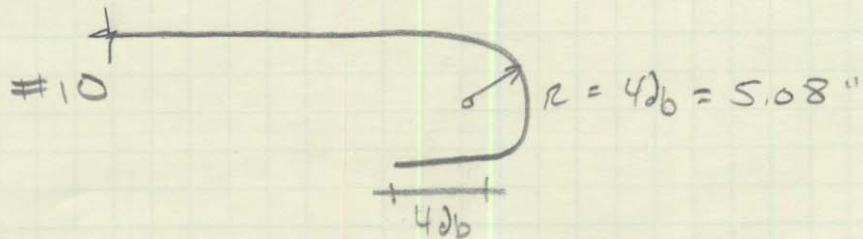
$\alpha = 1.3$ TOP BAR
 $\beta = 1.0$ NO EPOXY
 $\lambda = 1.0$ N.W.C.

$$l_{dh} = \frac{6000 (1.3)}{20 \sqrt{9000}} d_b = 42 (0.7) \approx 30 d_b$$

\uparrow
180° Mf

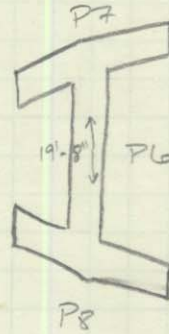
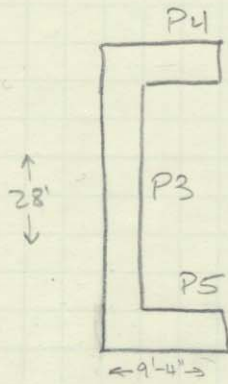
#10 — $l_{dh} = 53.3 \text{ IN } (0.7) \approx 40 \text{ IN}$

#11 — $l_{dh} = 59.22 \text{ IN } (0.7) \approx 45 \text{ IN}$



BOUNDARY &
TRANSVERSE REINFORCEMENT

BASE LEVEL



P3 - 1100 k
 P4 - 317 k
 P5 - 343 k ← USE THIS FOR P4

P6 - 876 k
 P7 - 328 k
 P8 - 350 k ← USE FOR P7

A_{gross} (OUT TO OUT)
 P3 - 37.33 SF
 P4/5 - 18.67

P6 - 26 SF
 P7/8 - 9.77 + 12.18
 33° 22°

CONCRETE
SHEAR CHECK

$$V_u < \phi 4 \sqrt{f'_c} A_v$$

P3: $1100 = \phi \underline{3.59} \sqrt{f'_c} A_v$

P4/5: $343 = \phi \underline{2.24} \sqrt{f'_c} A_v$

P6: $876 = \phi \underline{4.11} \sqrt{f'_c} A_v$, $4.11 \sim 4.0$

P7/8: $350 = \phi \underline{3.78} \sqrt{f'_c} A_v$

TRANSVERSE REINF

P3: $\rho = \frac{A_{st}}{A_{cv}}$

$A_{cv} = 12(16) = 192 \text{ in}^2/\text{ft}$

$A_{st} = .0025(192) = 0.48 \text{ in}^2/\text{ft}$

ASSUME 2#5 $A_{st} = 0.62 \text{ in}^2$

$$\frac{0.48}{12} = \frac{0.62}{s} = 15.5'' \text{ USE 2 CURTAINS \#5 @ 15'' OC}$$

P4/P5: $A_{cv} = 12(24) = 288$ $A_{st} = .0025 A_{cv} = 0.72 \text{ in}^2/\text{ft}$

ASSUME 2#5 $\frac{0.72}{12} = \frac{0.62}{s} \rightarrow s = 10.33'' \rightarrow 10'' \text{ OC}$

∴ USE 2 CURTAINS #5 @ 10'' OC FOR P4/5

USE 2 CURTAINS #4 @ 10'' OC FOR P3 $\left(\frac{0.48}{12} = \frac{A_s}{10} \right)$
 $A_s = 0.4 \text{ in}^2/\text{ft}$
 $\hookrightarrow 2\#4$

BOUNDARY &
TRANSVERSE REINF

P7/8: $A_{cu} = 288$ $A_{st} = 0.72 \text{ in}^2/\text{ft}$

USE (2) #5 @ 10" OC

SEE P3 &
P4/P5

P6: $A_{cu} = 192 \text{ in}^2/\text{ft}$ $A_{st} = 0.48 \text{ in}^2/\text{ft}$, USE (2) #4 @ 10" OC

CHECK 21.9.7.1 $V_u = A_{cu} (2\sqrt{f'_c} + \rho_t f_y)$, $\rho_t = \frac{V_u}{A_{cu} - 2\sqrt{f'_c}}$
 f_y

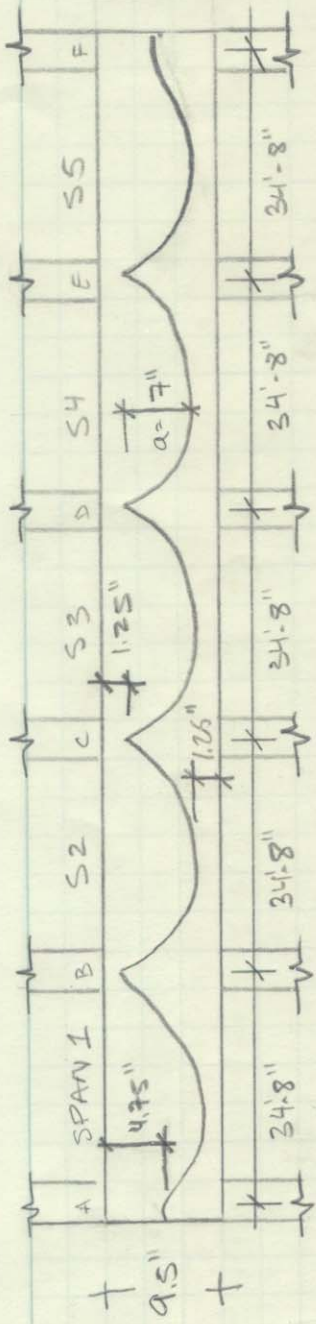
P3: $\rho_t = \frac{1100}{37.33 \times 144 - 2\sqrt{9000}} = -0.003 < \underline{0.0025}$
60000

P4/5: $\rho_t = \frac{343}{18.67 \times 144 - 2\sqrt{9000}} = -0.003 < \underline{0.0025}$
60000

P6: $\rho_t = \frac{876}{26 \times 144 - 2\sqrt{9000}} = -0.003 < \underline{0.0025}$
60000

P7/8: $\rho_t = \frac{350}{(9.77 \cos 33 + 12.18 \cos 22) 144 - 2\sqrt{9000}} = -3.4 \times 10^{-5} < \underline{0.0025}$
60000

$\rho_t = 0.0025$ OK IN ALL PIERS



$$T_{\text{SLAB}} = \frac{\text{SPAN}}{45} \approx \frac{35(12)}{45} = 9.33 \text{ IN} \rightarrow \text{USE } 9.5 \text{ IN FLAT SLAB}$$

$$\text{LOADS: SLAB + DEAD} = \frac{9.5}{12}(150) + 20 = 140 \text{ PSF}$$

$$\text{LIVE} = 40 \text{ PSF}$$

$$\text{PLF} = (140 + 40)(4 \text{ FT SPACING}) = 720 \text{ PLF}$$

BALANCE 85% DEAD = $0.85(140) = 119 \text{ PSF} \times 4' = 476 \text{ PLF}$
 USE $\frac{1}{2} \phi 270^{\circ}$ TENDONS, ASSUME 30% LOSSES

$$f_e = 0.153 \times (0.7)(270^{\circ} - 30^{\circ}) = 25.7 \text{ K PER TENDON}$$

$$F_e = 25.7 \times (4) / 4' = 25.7 \text{ K}$$

$$F/A = 25.7 / (9.5 \times 12) = 0.225 \text{ KSI} < 250 \text{ PSI INDUSTRY STANDARD}$$

$$\omega_{\text{BAL}} = \frac{F_e}{\rho_e^2} = \frac{8(25.7 \text{ K})(7 \text{ IN})}{(35 \text{ FT})^2} \times \frac{1}{12} = 0.098 \text{ KSF} \times 4 \text{ FT} = 0.392 \text{ KLF}$$

$$\omega_{\text{NET}} = 0.180 \text{ KSF} - 0.098 \text{ KSF}$$

$$= 0.082 \text{ KSF}$$

NET LOAD

$$\omega_{\text{NET}} = 180 - 119 = 61 \text{ PSF}$$

$$= 476 \text{ PLF}$$

USE 4 TENDONS

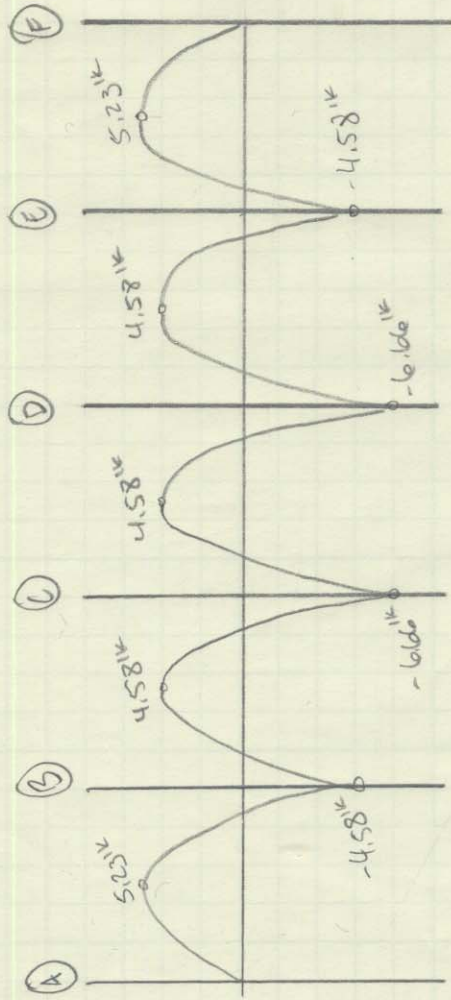
END MOMENTS

$$-M_{BA} = \frac{wL^2}{16} = 0.061(34.66)^2 = 4.58 \text{ k}$$

$$+M_{AB} = \frac{wL^2}{14} = 0.061(34.66)^2 = 5.23 \text{ k}$$

$$+M_{\text{ALL OTHER}} = \frac{wL^2}{16} = 4.58 \text{ k}$$

$$-M_{\text{ALL OTHER}} = \frac{wL^2}{11} = 6.66 \text{ k}$$



PT

Average Stresses: $A = 12 \times 9.5 = 114 \text{ in}^2$ $S = 2 \times 9.5^2 = 181 \text{ in}^3$

o NEGATIVE MOMENT

INTERIOR SPANS, $M_N^- = 6.66 \text{ k}$

$$f = \frac{F}{A} \pm \frac{M_N}{S} = \frac{25700}{114} \pm \frac{6660(12)}{181} = -225 \pm 442$$

$$= +217 < 380 = \sqrt{f_c}$$
$$-667 < 0.5 \times 4000 = 1200 \text{ PSI}$$

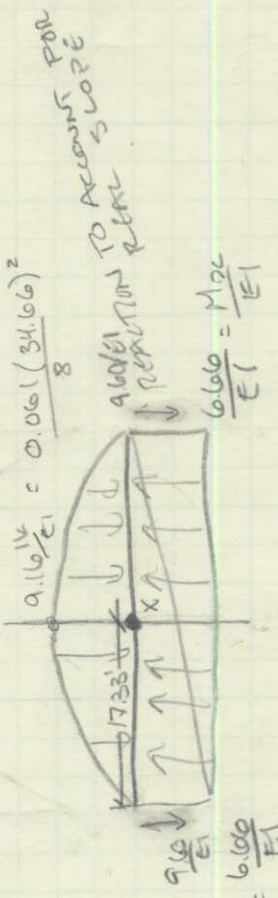
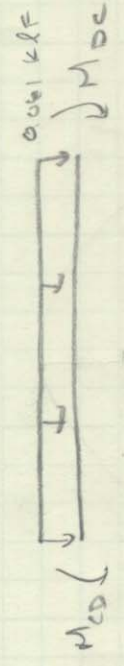
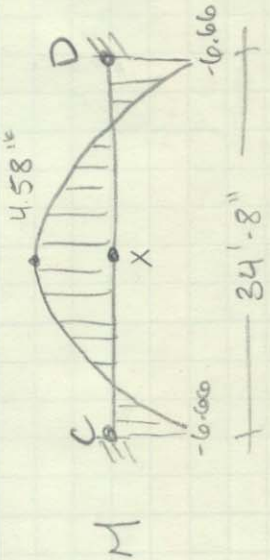
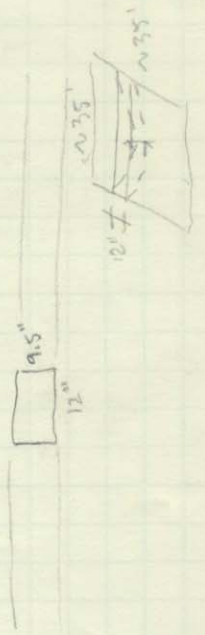
o POSITIVE MOMENT

INTERIOR SPANS, $M_N^+ = 4.58 \text{ k}$

$$f = \frac{-225 \pm 4580(12)}{181} = -225 \pm 304 = +79 < 2\sqrt{f_c} = 126 \text{ PSI}$$
$$= -529$$

④ DEFLECTION: CONJUGATE BEAM

$E = 57\sqrt{4000} = 3605 \text{ psi}$
 $I = \frac{b h^3}{12} = \frac{12 (9.5)^3}{12} = 857 \text{ in}^4/\text{ft}$



REACTION:
 $\sum A: \frac{2}{3} (34.66) (9.16) - \frac{6.66 (34.66)}{EI} = 0$
 $\sum F_y: -19.18 + 2R = 0$
 $R_{\text{REACTION}} = -19.18/EI$
 $= -9.60/EI$

$\sum M_x =$
 MID SPAN DEF.: $\left[\frac{2}{3} (9.16) (17.33) \left(\frac{2}{3} (17.33) \right) - \frac{6.66 (17.33) (17.33)}{EI} - \frac{9.60 (17.33)}{EI} \right]$

$\sum M_x = \Delta_x = -14.6/EI - 479/EI$

$\Delta_x = -14.6 (FT/K) (3605 \frac{lb}{in^2}) (857 \frac{in^4}{ft})$

$(1728) = -0.268 \text{ in}$

DEFL CONJ BEAM

SPAN AB: M

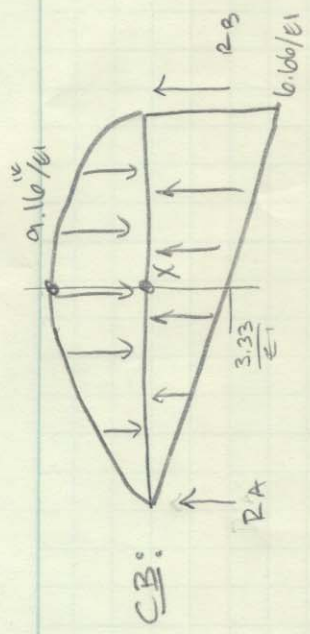


$E = 3605 \text{ ksi}$
 $I = 857 \text{ in}^4/\text{ft}$

MID SPAN DEF

$$\sum M_x = \left[\left(\frac{2}{3} \right) (9.16) (17.33) \right] - \left[3.33 \left(\frac{1}{2} \right) (17.33) \right] \left(17.33 \right) - 67.3 \left(17.33 \right)$$

$$= -\frac{645}{E I} = -\frac{645 (857) (1728)}{3605 (857)} = -0.361 \text{ IN}$$



$$\sum F_x = -\frac{2}{3} (9.16) (34.67) + 6.66 (34.67) \left(\frac{1}{2} \right)$$

$$= -96.2/EI$$

$$\sum M = -R_A (34.67) + \left[9.16 \left(\frac{2}{3} \right) (34.67) \right] (17.33) - \frac{6.66 (34.67) (34.67)}{3}$$

$$R_A = \frac{67.31k}{E I} \quad R_B = \frac{28.91k}{E I}$$

$$9.16 = \frac{(0.061) (34.67)^2}{8}$$

ULTIMATE STR

SPAN CD $+M_{CD} = 4.58 \text{ KFT/FT}$ $-M = 6.66 \text{ KFT/FT}$

ESTIMATE: $\frac{4.58}{4.3(8.25)} = 0.129 \text{ in}^2/\text{FT} \rightarrow \text{T24 \#5 AT } 18'' A_s = 0.21 \text{ in}^2/\text{FT}$

$$a = \frac{0.21(60)}{.85(f'_c)b} = \frac{0.21(60)}{.85(4)(12)} = 0.31 \text{ in}$$

$$\phi M_n = 0.9(0.21)(60)(8.25 - \frac{0.31}{2}) \frac{1}{12} = 7.65 \text{ KFT/FT}$$

#5 @ 24" $A_s = 0.155 \text{ in}^2/\text{FT}$

$$a = \frac{0.155(60)}{.85 f'_c b} = 0.23$$

$$\phi M_n = 0.9(0.155)(60)(8.25 - \frac{0.23}{2}) \frac{1}{12} = 5.67 \text{ KFT/FT}$$

USE #5 @ 24", M+

ESTIMATE: $\frac{6.66}{4.3(8.25)} = 0.19 \text{ in}^2/\text{FT}$ T24 #5 @ 18" $A_s = 0.21 \text{ in}^2/\text{FT}$

$a = 0.31 \text{ in}$ $\phi M_n = 7.65 \text{ KFT/FT}$ ✓ OK

#5 @ 20" $A_s = .186 \text{ in}^2/\text{FT}$

$$a = \frac{.186(60)}{.85(4)(12)} = 0.27$$

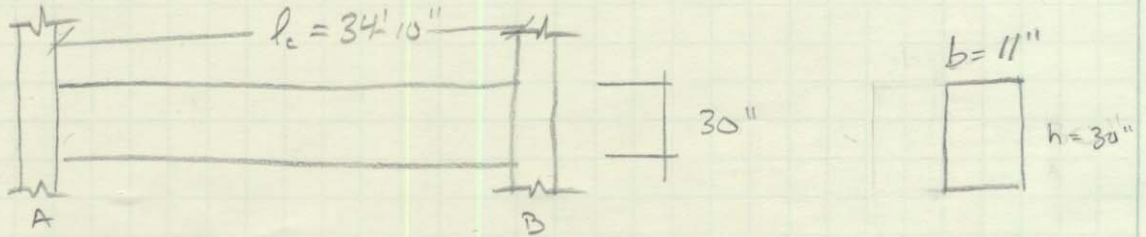
$$\phi M_n = 0.9(.186)(60)(8.25 - \frac{.27}{2}) \frac{1}{12} = 6.79 \text{ KFT/FT}$$

USE #5 @ 20" IN, M-

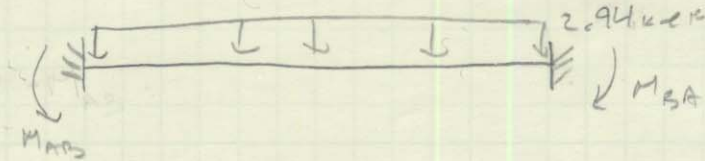
LONG TERM DEF

ASSUME: ○ BEAM AREA LOAD = 100 PSF

- ② TRIB WIDTH NORMALIZED AS 32.7 FT
- ③ FIXED END CONDITIONS



$$w = (90 \text{ PSF})(32.7 \text{ FT}) = 2.94 \text{ KRF}$$



$$M_{AB} = -\frac{wl^2}{12} = \frac{2.94(34.83)^2}{12} = 296 \text{ KFT}$$

$$M_{BA} = \frac{wl^2}{24} = \frac{296}{2} = 148 \text{ KFT}$$

STEEL

TOP: $\frac{M_u}{\phi_b} = \frac{330}{4.3(27)} = 2.84 \text{ IN}^2 \quad \text{T24 } 3\#9$

$$a = \frac{A_s f_y}{.85 f'_c b} = \frac{3.0(60)}{.85(4)(11)} = 4.81$$

$$\phi M_n = 0.9(3)(60)(27 - \frac{4.81}{2}) \frac{1}{12} = 332 \text{ FTK} \quad \checkmark \text{ OK}$$

$$A_{s \text{ MIN}} = \frac{3 \sqrt{f'_c} b d}{f_y} = \frac{3 \sqrt{4000} (11)(27)}{60000} = 0.93 \text{ IN}^2$$

$$\frac{200 b w d}{f_y} = \frac{200 (11) 27}{60000} = 0.99 \quad \checkmark \text{ OK}$$

BOTTOM: $\frac{M_u}{\phi_b} = \frac{165 \text{ KFT}}{4.3(27)} = 1.42 \text{ IN}^2 \quad \text{T24 } 2\#9$

$$a = \frac{A_s f_y}{.85 f'_c b} = \frac{2(60)}{.85(4)(11)} = 3.21 \text{ IN}$$

$$\phi M_n = .9(2.0)(60)(27 - \frac{3.21}{2}) \frac{1}{12} = 228 \text{ FTK} \quad \checkmark \text{ OK}$$

L.T.D.

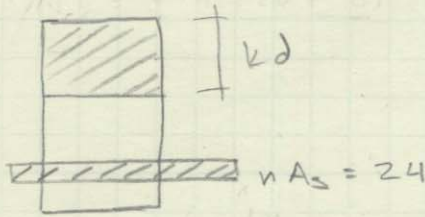
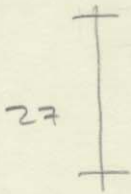
$$E_c = 3.1 \times 10^4 \text{ ksi}$$

$$E_s = 29,000 \text{ ksi}$$

$$n = 8$$

$$f_r = 7.5 \sqrt{4000} = 474 \text{ psi}$$

$$I_g = 24,750 \text{ in}^4$$



$$M_{cr} = 474 \left(\frac{24,750}{15} \right) \times \frac{1}{12,000} = 65.2$$

$$M_{cr}/M_a = 0.44$$

$$E_d = \frac{kd(b)(kd/2) + nA_s(d)}{bkd + nA_s} = \frac{\frac{11}{2}kd^2 + 24(27)}{11kd + 24}$$

$$11kd^2 + 24kd = 5.5kd^2 + 648$$

$$5.5kd^2 + 24kd - 648 = 0 \quad kd = 8.9$$

$$I_{cr} = \frac{b(E_d)^3}{3} + nA_s(d - kd)^2$$

$$= \frac{11(8.9)^3}{3} + 24(27 - 8.9)^2 = 10,447 \text{ in}^4$$

$$I_e = 0.44^3 \times 24,750 + (1 - 0.44^3) 10,447 = 11,665$$

$$\Delta = \frac{5}{384} \frac{w_s w_s l^4}{E_c I_e} = \frac{5}{384} \frac{2.94 (34.83)^4 (1728)}{(3600)(11,665)} = 2.31 \text{ in}$$

$$\lambda = \frac{\Delta}{1 + 50\rho'} = \frac{2}{1 + 50(0.01)} = 1.33$$

$$\Delta_{sus} = 1.33(2.31) = 3.08 \approx l/136 \text{ More steel}$$

$$\text{TR4 } 4 \# 10 \quad A_s = 5.08 \text{ in}^2$$

$$\frac{b}{2} kd^2 + nA_s kd - nA_s d = 0$$

$$\frac{11}{2} kd^2 + 40.64(kd) - 1098 = 0 \quad kd = 10.9$$

$$I_{cr} = \frac{11(10.9)^3}{3} + 40.64(27 - 10.9)^2 = 15,282$$

$$I_e = 0.44^3 \times 24,750 + (1 - 0.44^3) 15,282 = 16,100 \text{ in}^4$$

$$\Delta = \frac{5}{384} \frac{2.94 (34.83)^4 (1728)}{3600 (16,100)} = 1.68 \text{ in} \quad \Delta_{sus} = \frac{2}{1 + 50(0.017)} = 1.68$$

$$= 1.81 \approx l/231$$

$$\text{TR4 } b = 14'' \text{ w/ } 4\#11 \text{ } A_s = 6.24 \text{ in}^2$$

$$I_g = 14 \frac{(30)^3}{12} = 31500$$

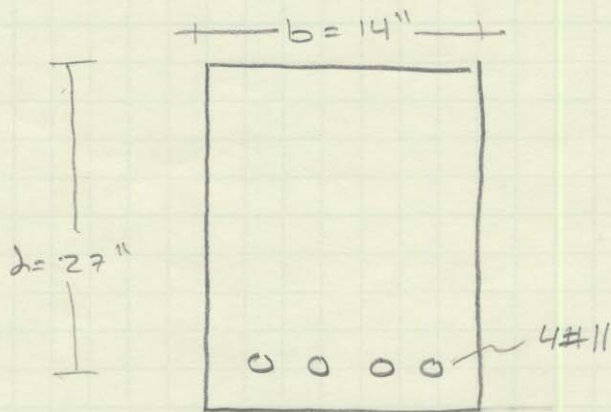
$$\frac{14}{2} kd^2 + 8(6.24)kd - 1348 = 0 \quad kd = 10.8 \text{ in}$$

$$I_{cr} = 14 \frac{(10.8)^3}{3} + 49.9(27 - 10.8)^2 = 18980 \text{ in}^4$$

$$I_e = .44^3(31500) + (1 - .44^3)(18980) = 20050$$

$$\Delta = \frac{5}{384} \frac{2.97(34.83)^4}{2600(20050)} 1728 = 1.36 \text{ in}$$

$$\Delta_{sus} = 1.50 \sim 2/280 \text{ OKAY}$$



APPROG - JAN 07 ~ 47 WEEKS

- 50 PRL CREW FOR FLOOR PLATE MAX

FLOOR - 10600 SF

OLD - 5707 CY MI - LOCATION FACTOR = 83.3

NEW - 7228 CY → 6086 (8" SLAB) CA - Loc. Factor = 121.8

OLD 2100 FLAT PLATE, 125 PPSUP LOAD, 15' SPAN

CREW C-14B → 28 WORKERS

D.O. - 30.24 CY

L.H. - 6.878 / CY TOTAL

COST = 5707 CY (\$519 / CY) = \$2,960,000

SCHEDULE = 5707 CY / 30.24 CY / DAY = 189 DAYS $\frac{\text{CY}}{\text{DAYS}} = \frac{38 \text{ WEEKS}}{5 \text{ DAYS}} < 47 \text{ W}$

NEW 03 41 36.50 POST TENSIONE JOBS

0200 LARGE JOB

CREW C-17B → 10.75 WORKERS

D.O. - 10 CY

L.H. - 8.200 / CY

COST = 7230 CY (\$821 / CY) = \$5,930,000 w/ 1 CREW

SCH = 7230 CY / 10 CY / DAY = 723 DAYS / 2.5 CREWS = 289 DAYS = 58 WEEKS
/ 3 CREWS = 241 DAYS = 48 WEEKS



COST ANALYSIS

COST ANALYSIS

OLD 3/4 CREW → 24 WORKERS

DO: $30.24 (.75) = 22.68 \text{ CY}$

SCH: $5707 \text{ CY} / 22.68 \text{ CY/DAY} = 250 \text{ DAYS} = 50 \text{ WEEKS} \approx 47 \text{ WEEKS ACTUAL}$

COST: $5707 \text{ CY} (\$242 + \frac{\$0.95(255)}{24} + \$24) = \$2,600,000$
MATERIAL LABOUR EPP

NEW 2.5x CREW

COST: $7230 \text{ CY} (\$475 + 2.5(315) + \$31) = \$9,350,000$

SCH: $7230 \text{ CY} / (2.5 \times 10 \text{ CY/DAY}) = 289 \text{ D} = 58 \text{ WEEKS} > 47 \text{ W}$

NEW 3x CREW

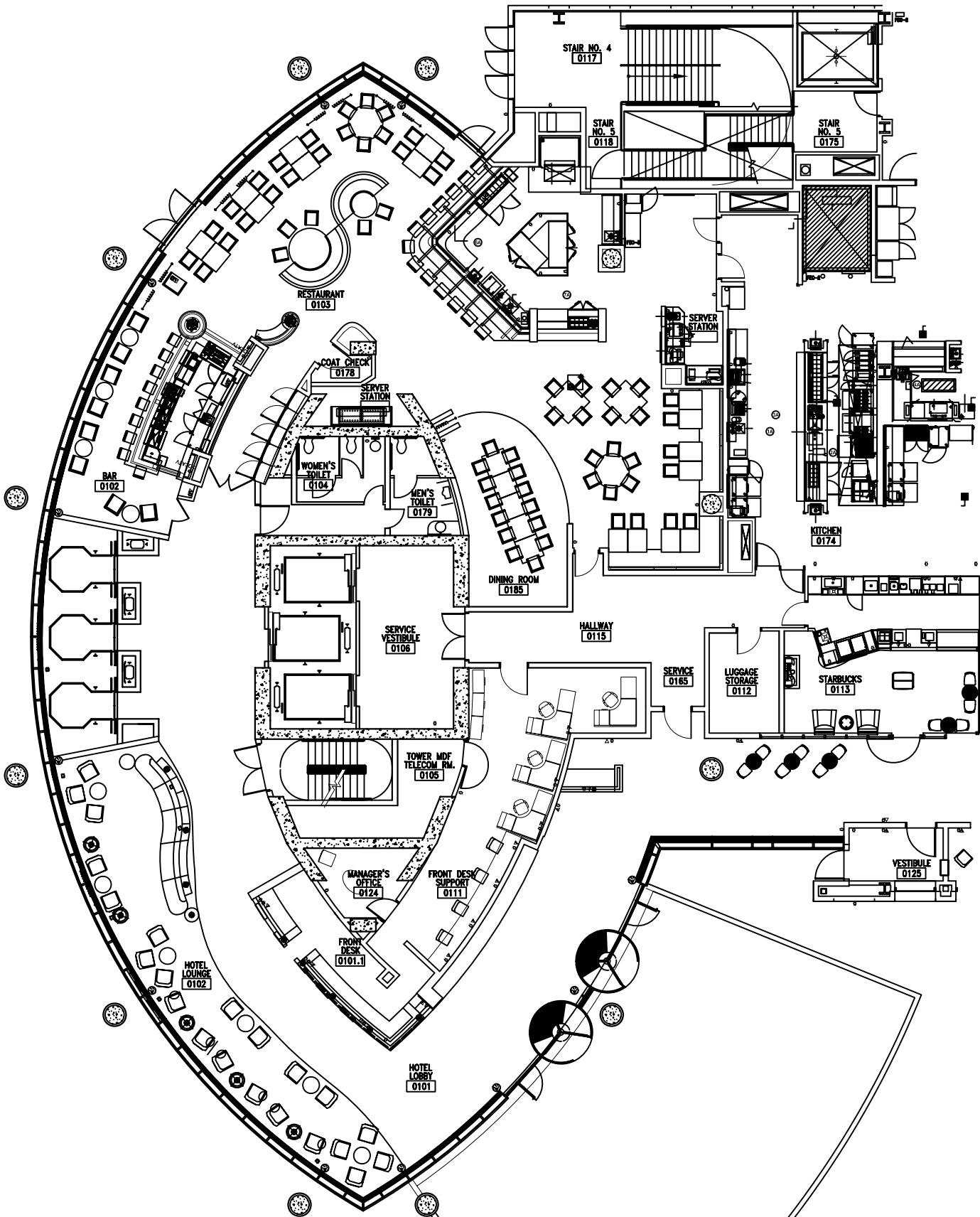
COST: $7230 \text{ CY} (475 + 3.0(315) + 31) = \$10,490,000$

SCH: $7230 / 3.0 \times 10 \text{ CY/DAY} = 240 \text{ D} = 48 \text{ WEEKS} \approx 47 \text{ W}$

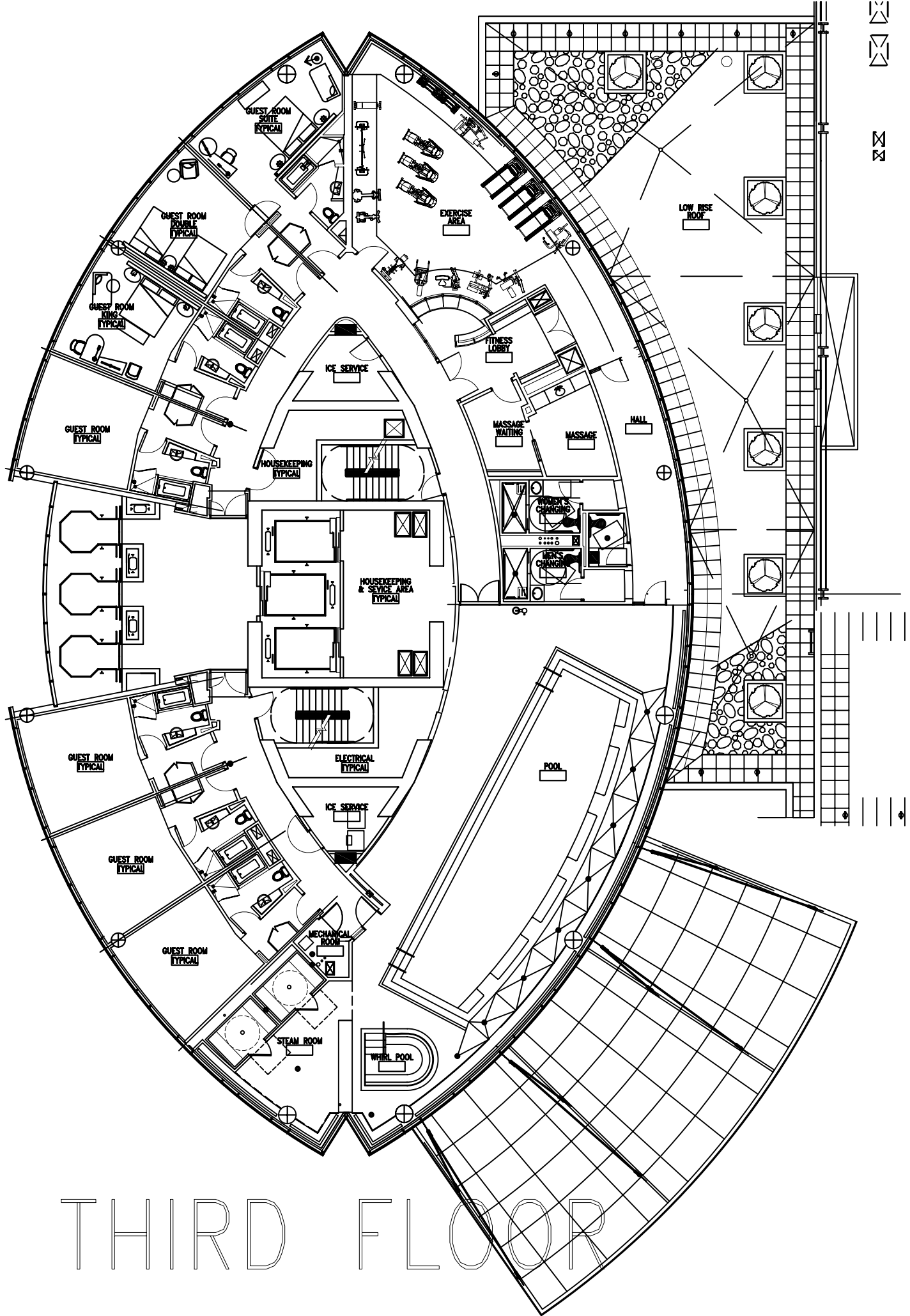


APPENDIX C: NEW FLOOR PLANS

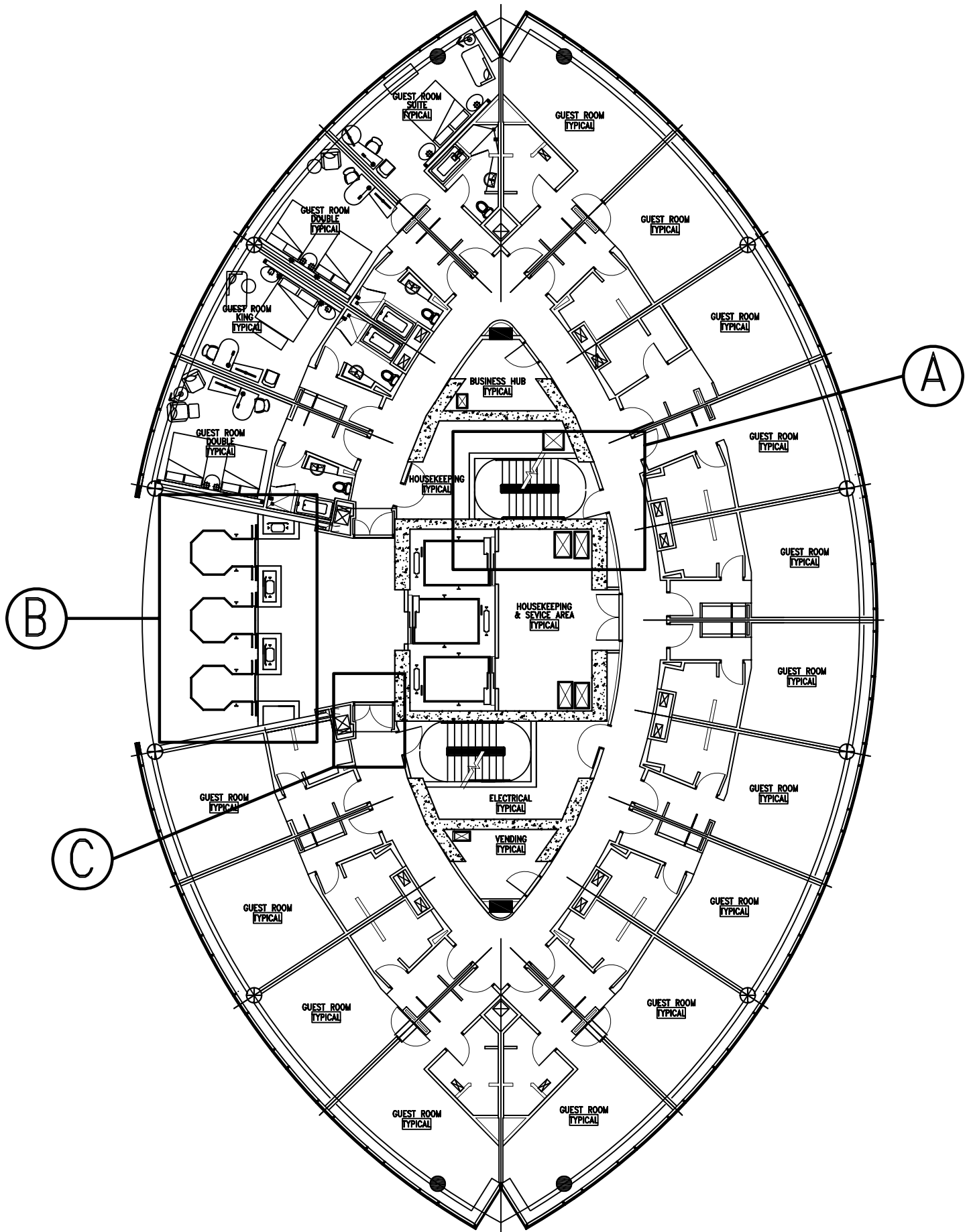




GROUND FLOOR



THIRD FLOOR



TYPICAL FLOOR